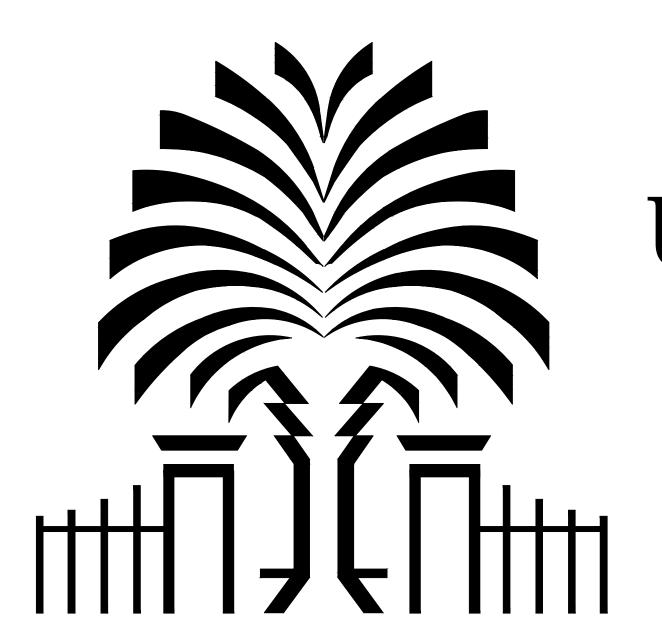
# USCA Pedestrian Bridge

State Project No.: H29-9545-PG In Aiken County, SC



Prepared For University of South Carolina Aiken

Chao & Associates, Inc.

Consulting Engineers

C & A #: 391402B-11

October 9, 2012

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UNIVERSITY OF SOUTH CAROLINA AIKEN

FACILITIES MANAGEMENT 471 UNIVERSITY PARKWAY

AIKEN. SC 29801

PROJECT MANAGER,

CIVIL, SURVEYING, AND STRUCTURAL ENGINEER:

CHAO & ASSOCIATES, INC 7 CLUSTERS COURT

COLUMBIA, SC 29210

TEL: 803-772-8420

ARCHITECT:

McCREARY/SNOW ARCHITECTS, PA

PO BOX 11143

COLUMBIA, SC 29211

TEL: 803-771-6267

**ELECTRICAL ENGINEER:** 

GWA, INC

**168 LAURELHURST AVENUE** COLUMBIA, SC 29210

TEL: 803-252-6919

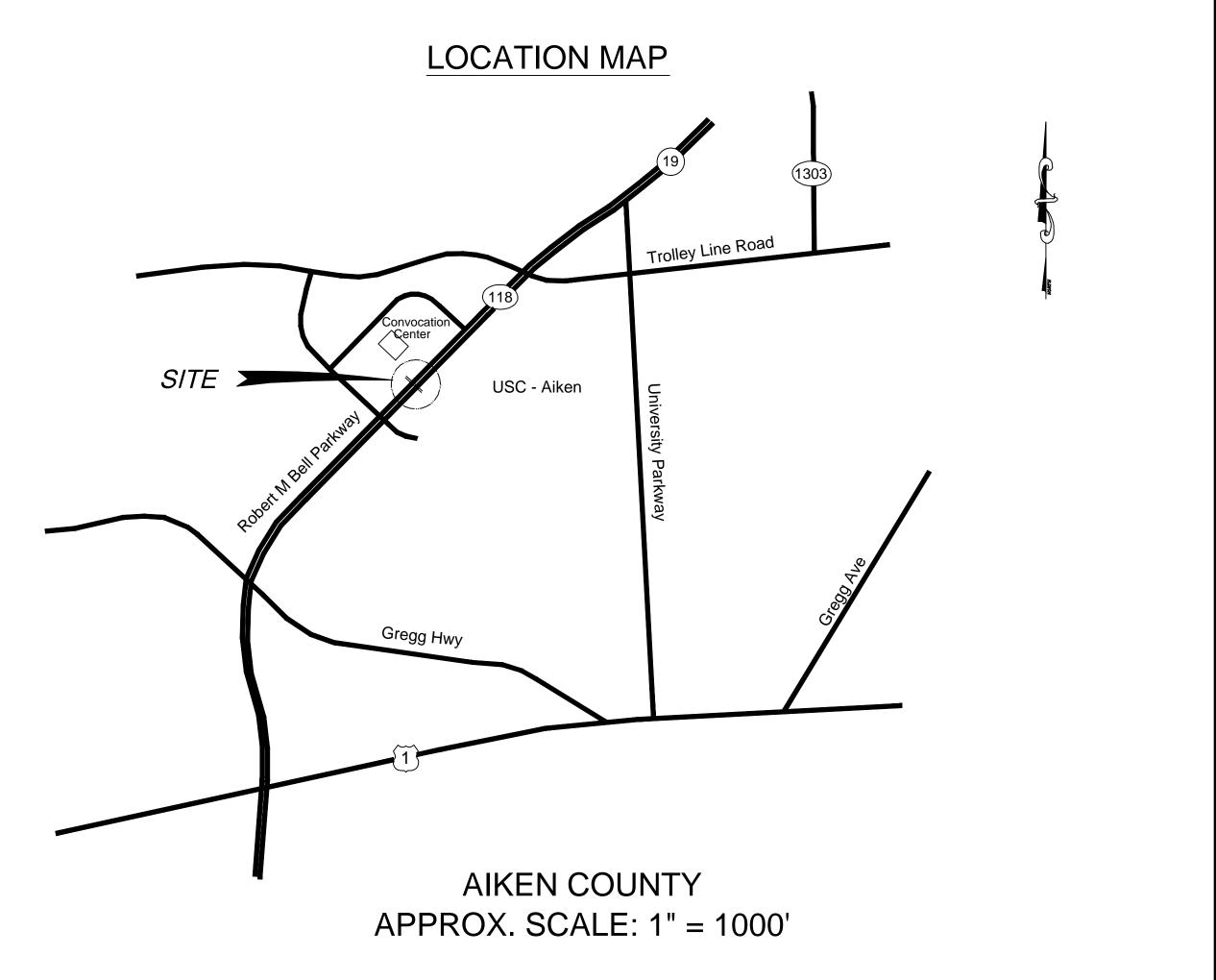
**GEOTECHNICAL ENGINEER:** 

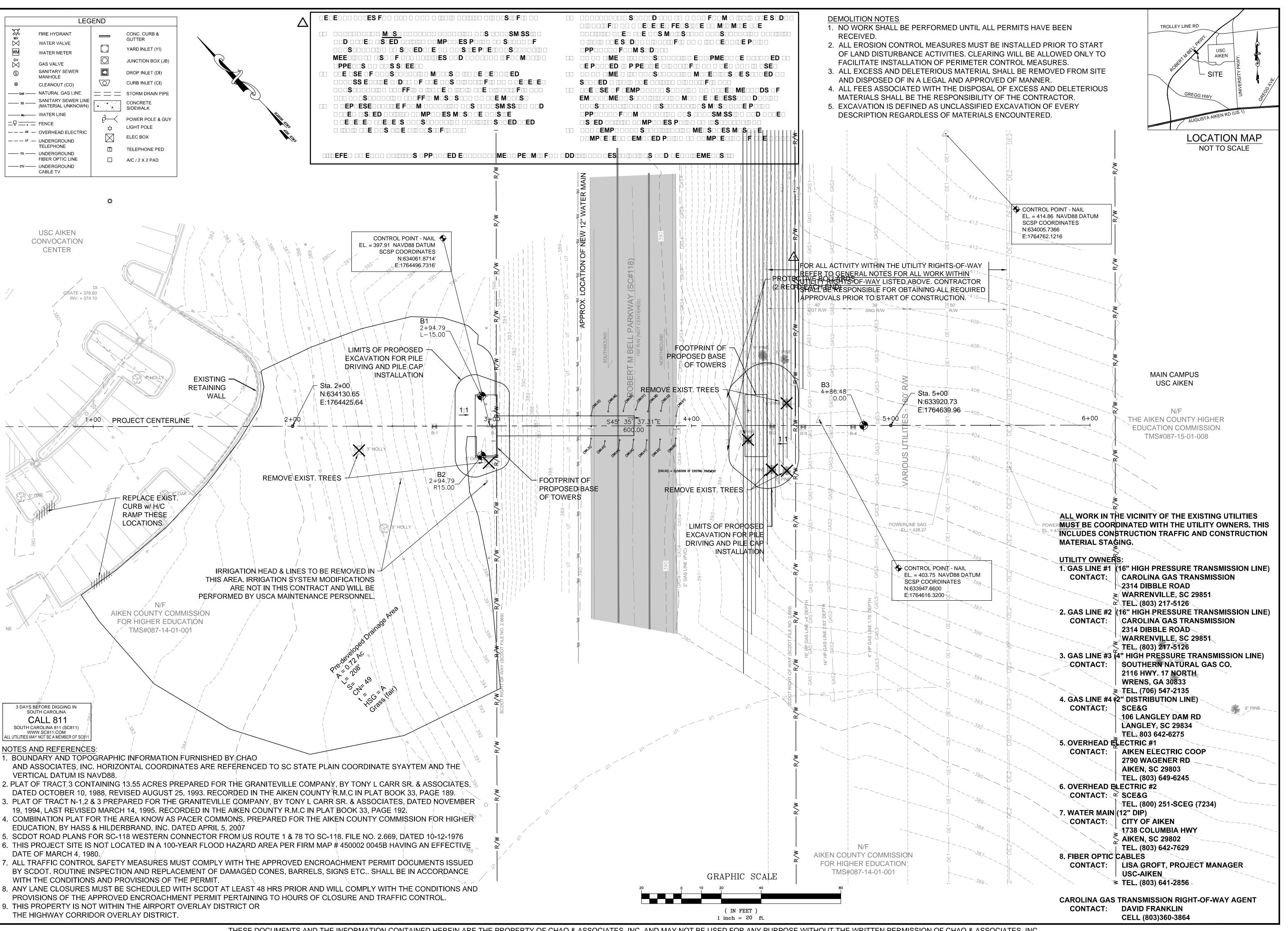
GEOSTELLAR ENGINEERING, LLC **1077 RODNEY DRIVE** 

BATON ROUGE, LA 70808 TEL: 803-997-0554







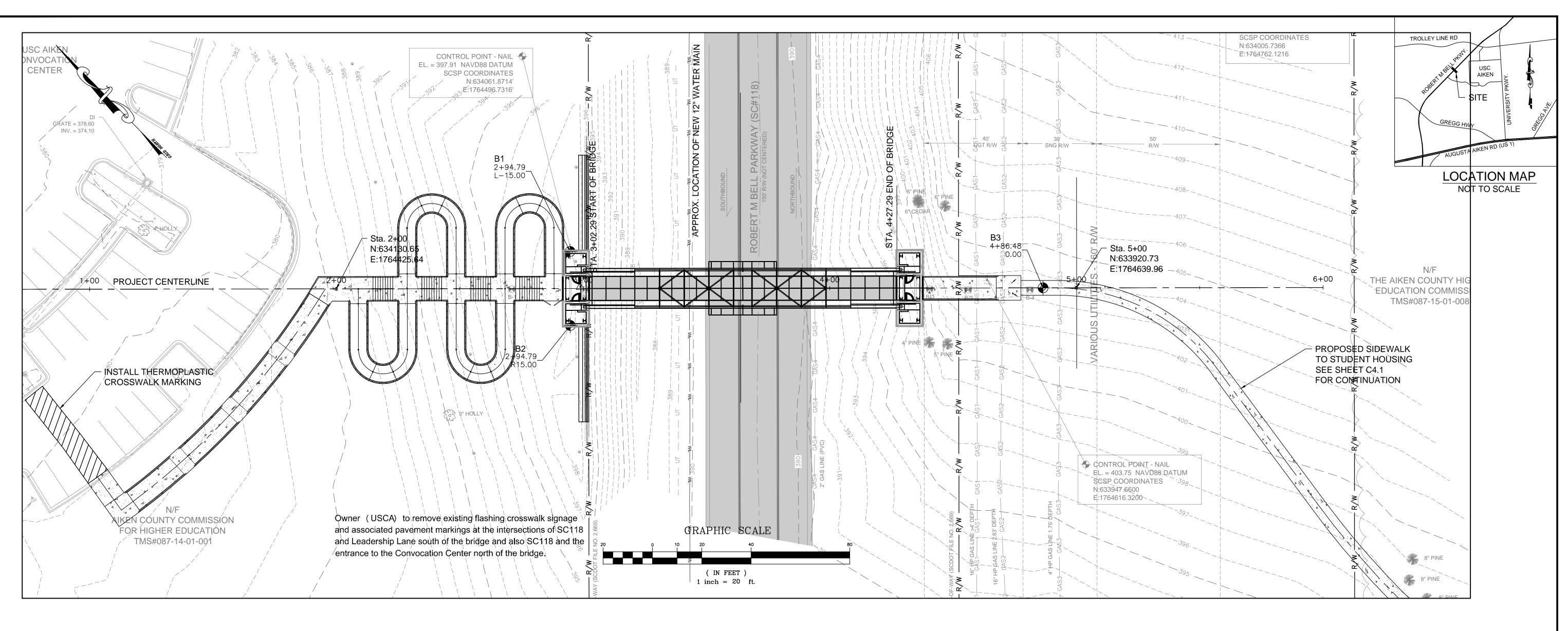


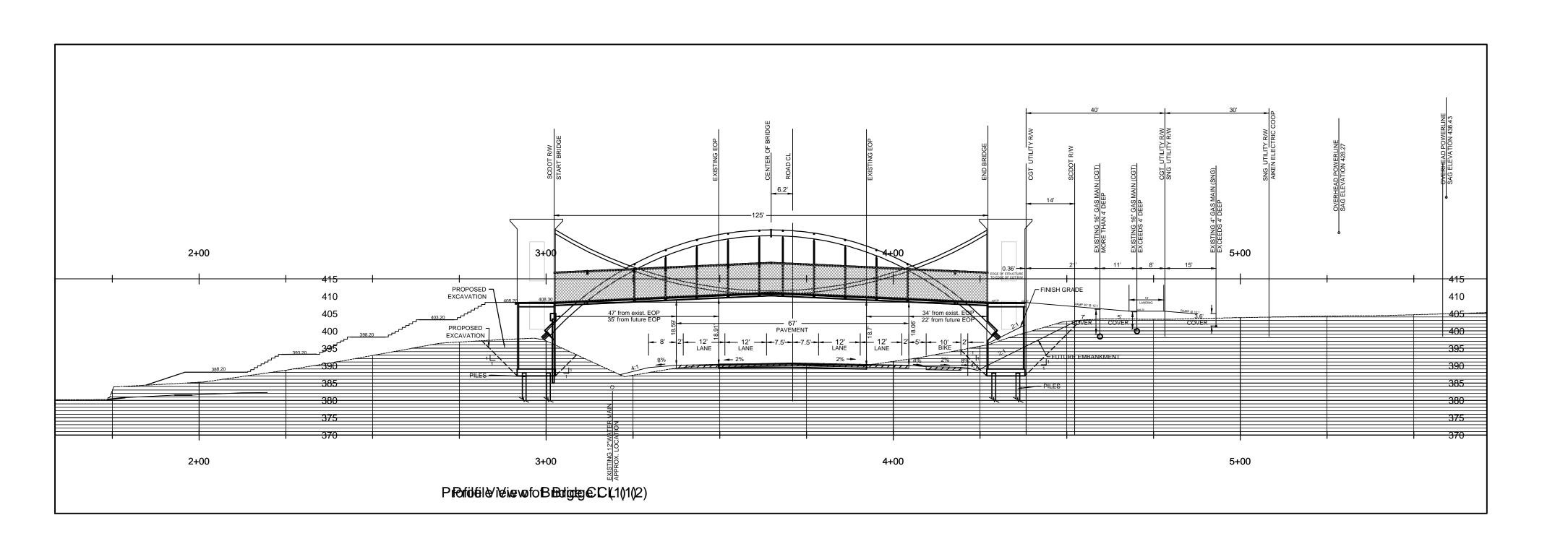
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ASSOCIATES No. C00357

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NOTES:

3 DAYS BEFORE DIGGING IN SOUTH CAROLINA

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- 2. BENCHMARK IS BASED ON MEAN NAVD88 DATUM.
- 3. PROPOSED CONTOURS & PROPOSED SPOT ELEVATIONS REPRESENT

FINISHED GRADE.

4. THIS PROPERTY IS NOT LOCATED IN THE AIRPORT OVERLAY DISTRICT OR HIGHWAY CORRIDOR OVERLAY DISTRICT.

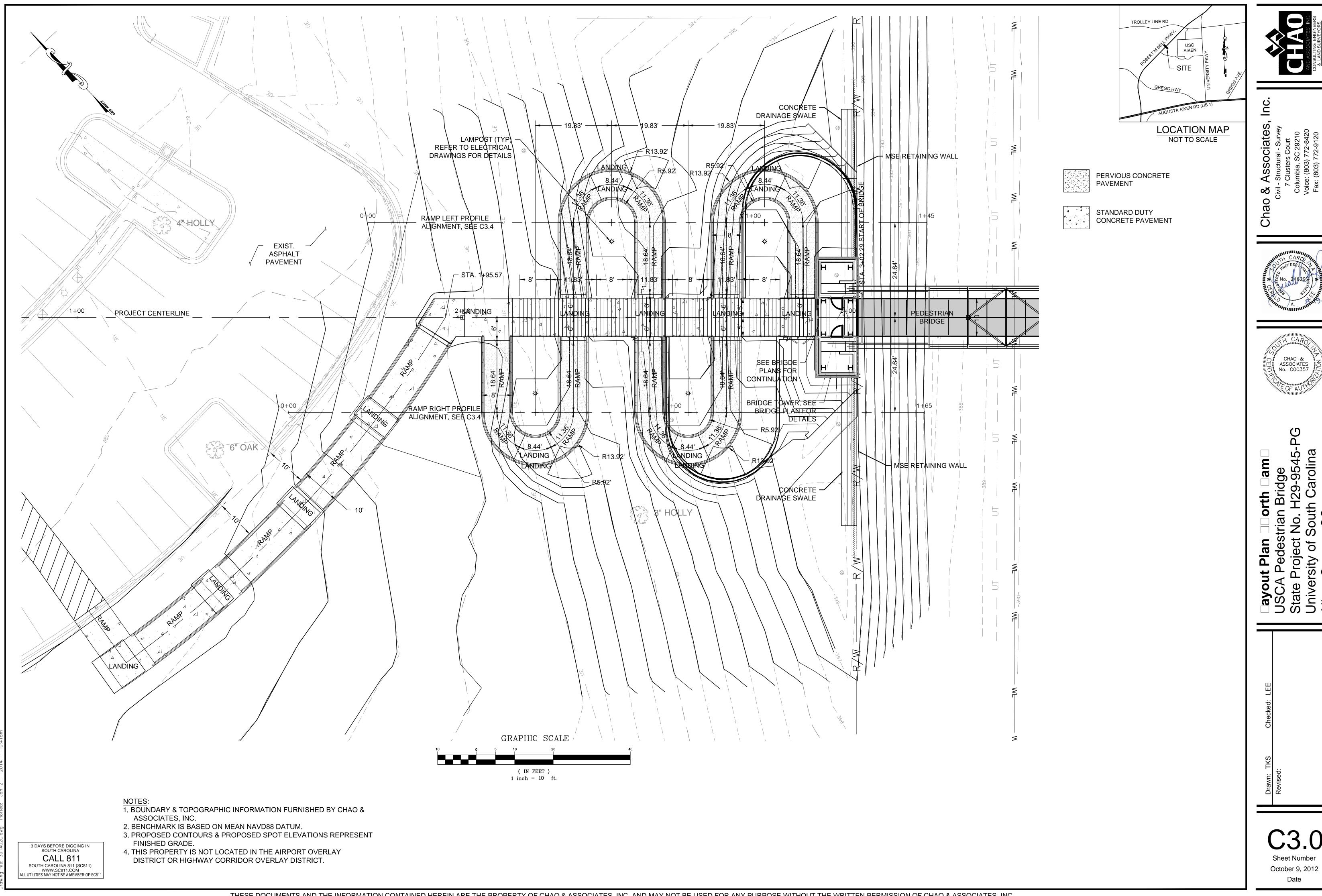
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Associates,

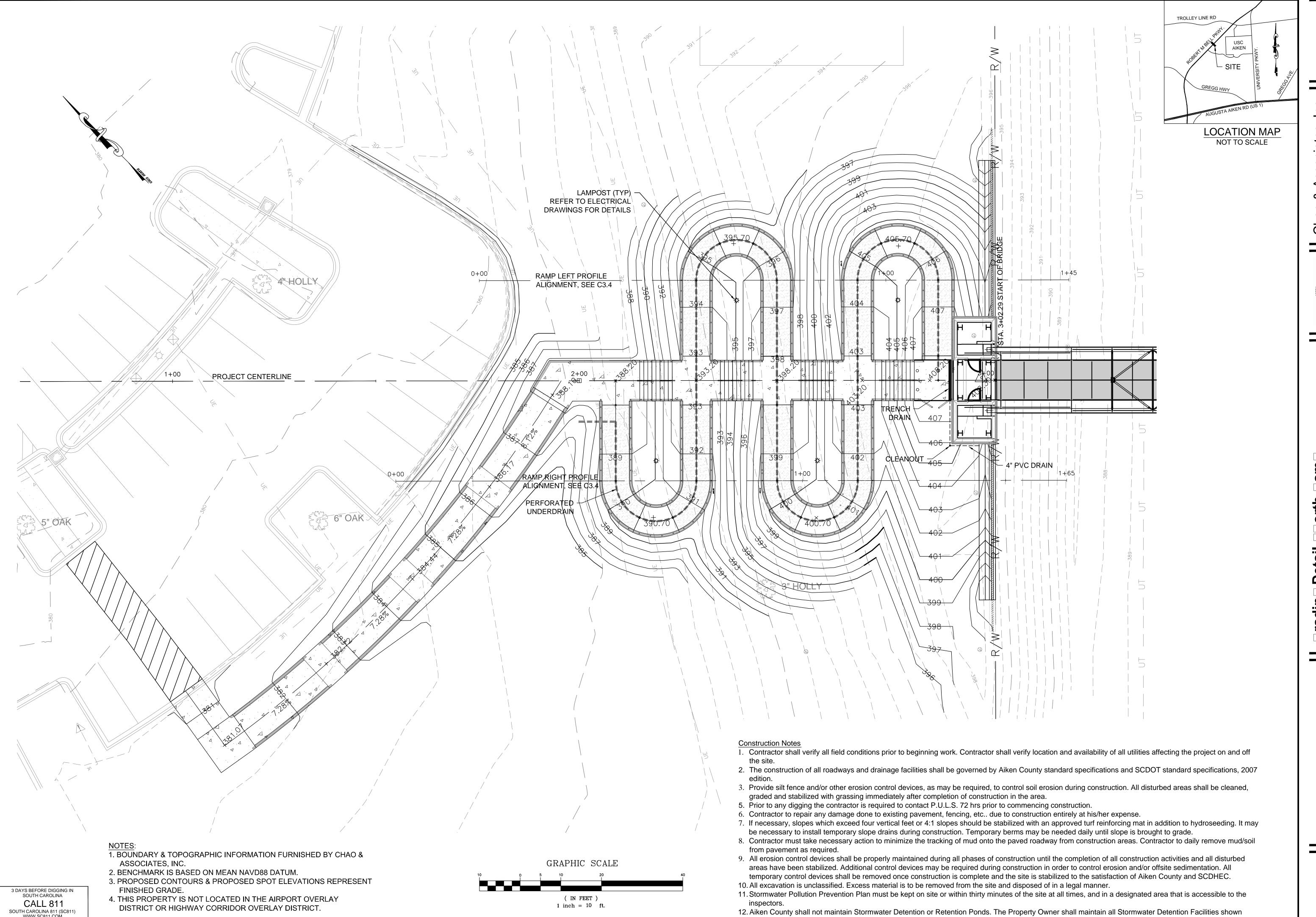
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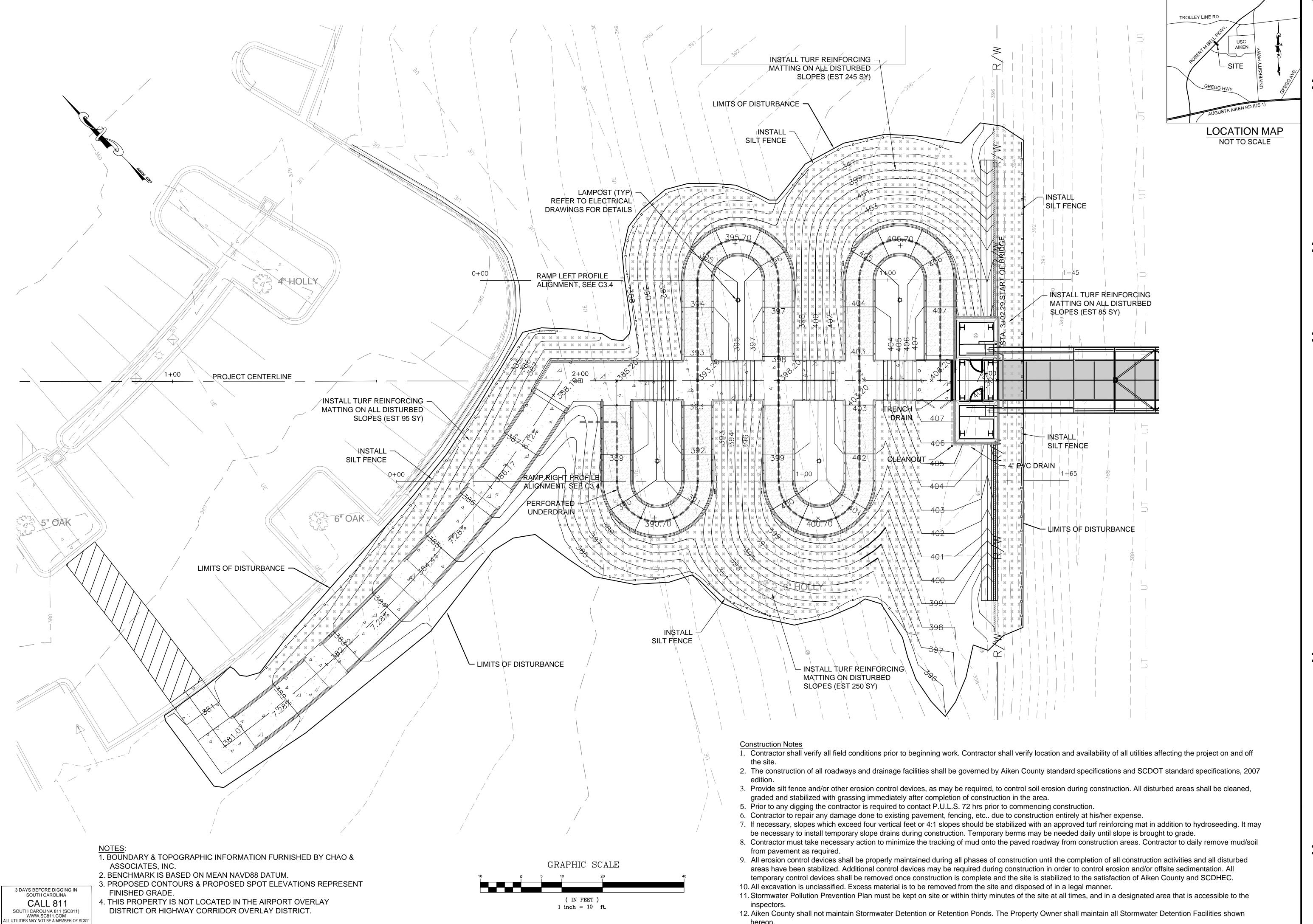
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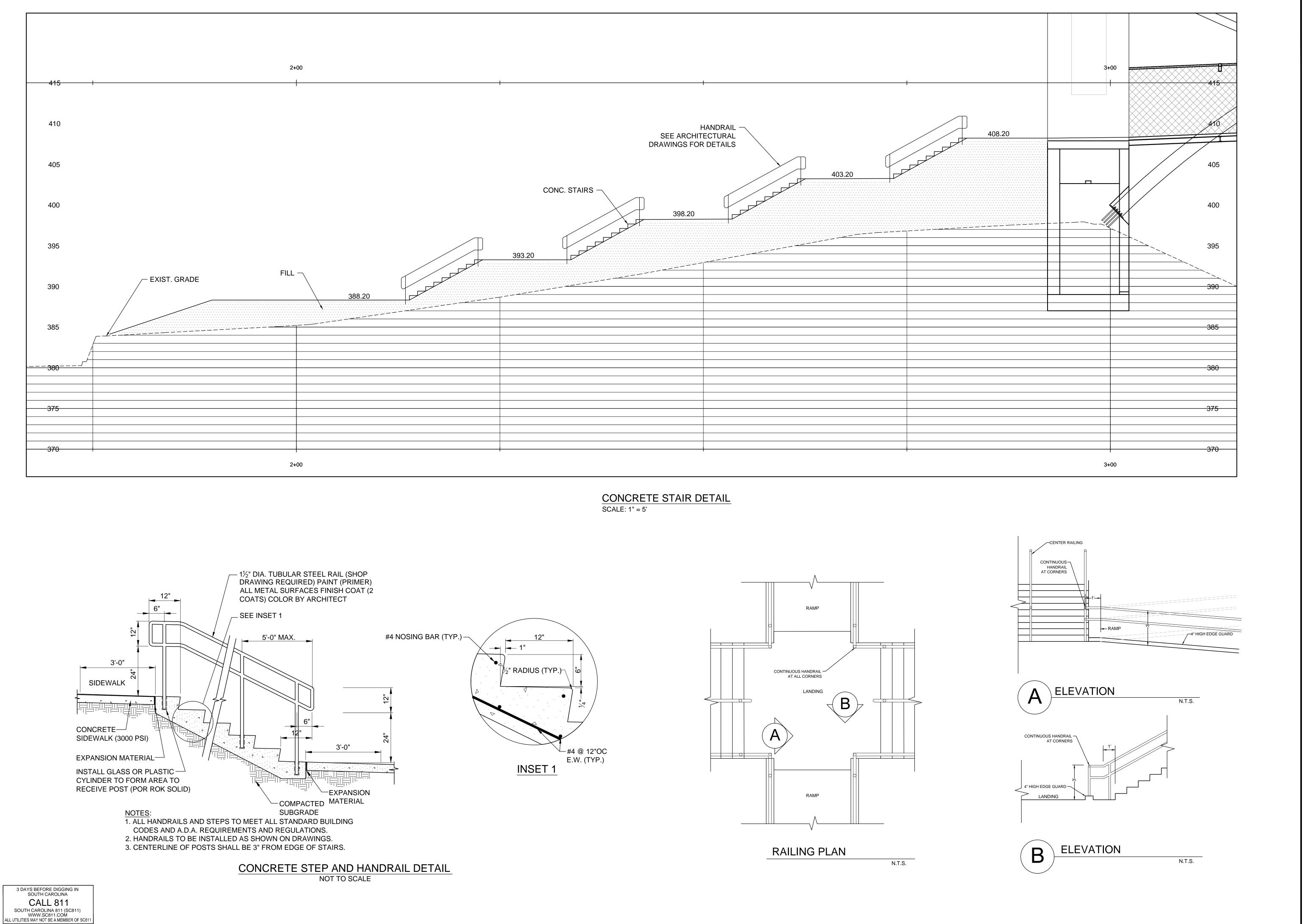




Associates

CHAO & ASSOCIATES

October 9, 2012 Date



ANE ASSOCIATES INC.
CONSULTING ENGINEERS
& LAND SURVEYORS

O & ASSOCIATES, INC Civil - Structural - Survey 7 Clusters Court Columbia, SC 29210 Voice: (803) 772-8420

Colum Voice:

No. 21629 E

CHAO & SSOCIATES No. CO0357

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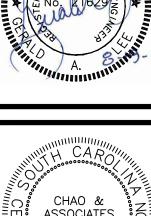
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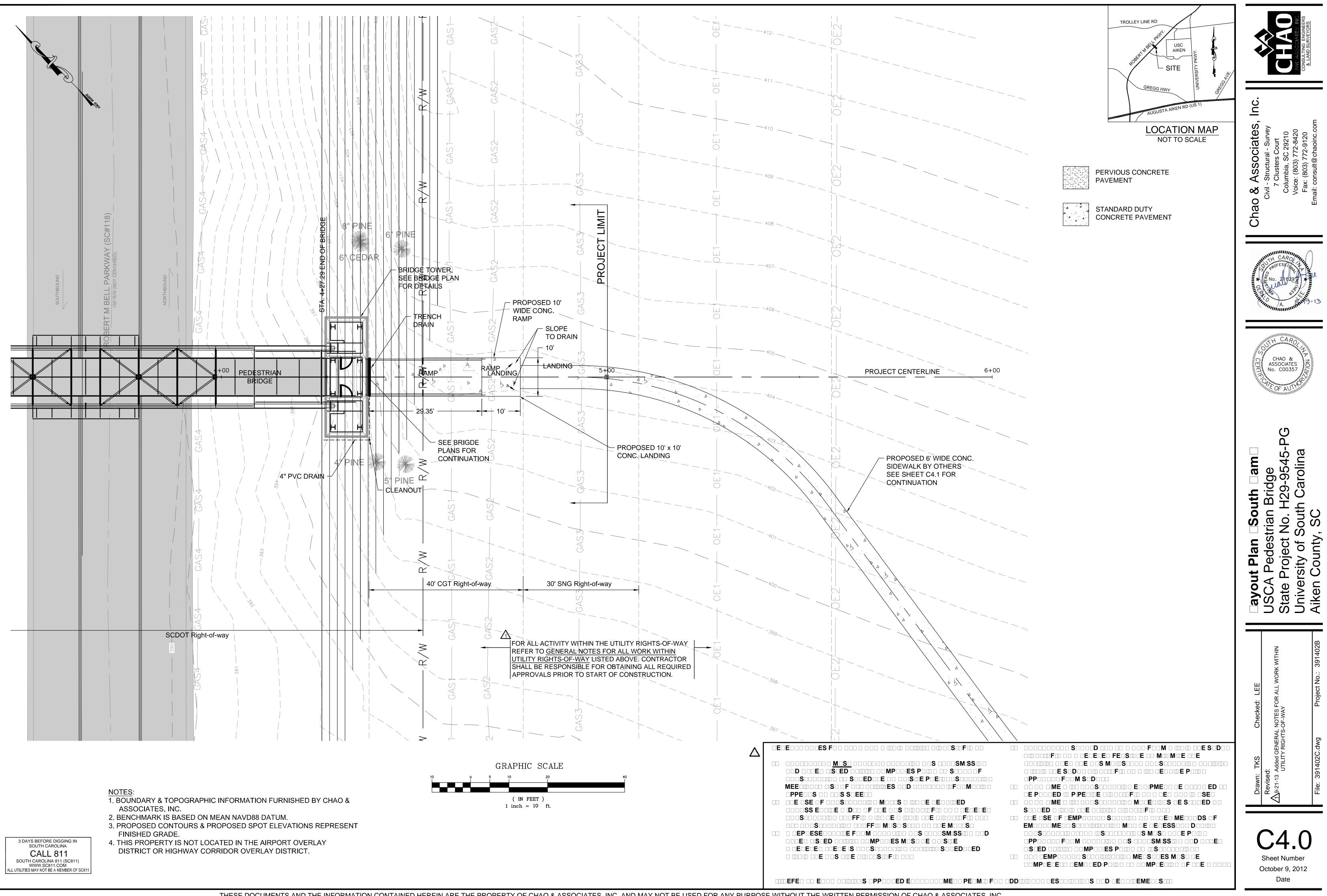
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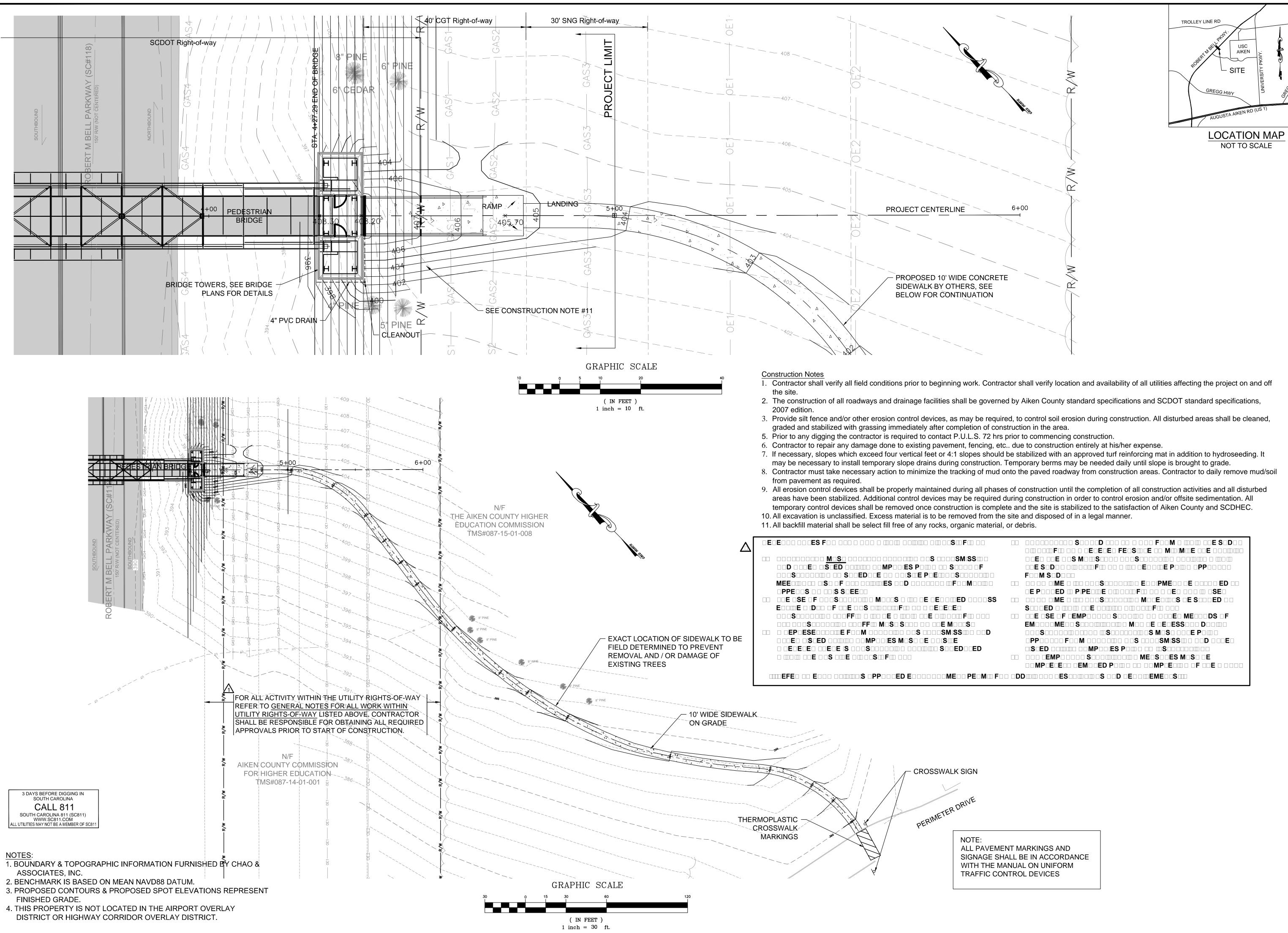






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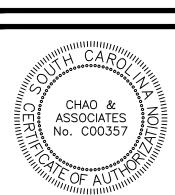


Structural - Survey Clusters Court Imbia, SC 29210 3: (803) 772-8420

Associates

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CIVII - Structural - Su 7 Clusters Court Columbia, SC 292 Voice: (803) 772-84 Fax: (803) 772-913



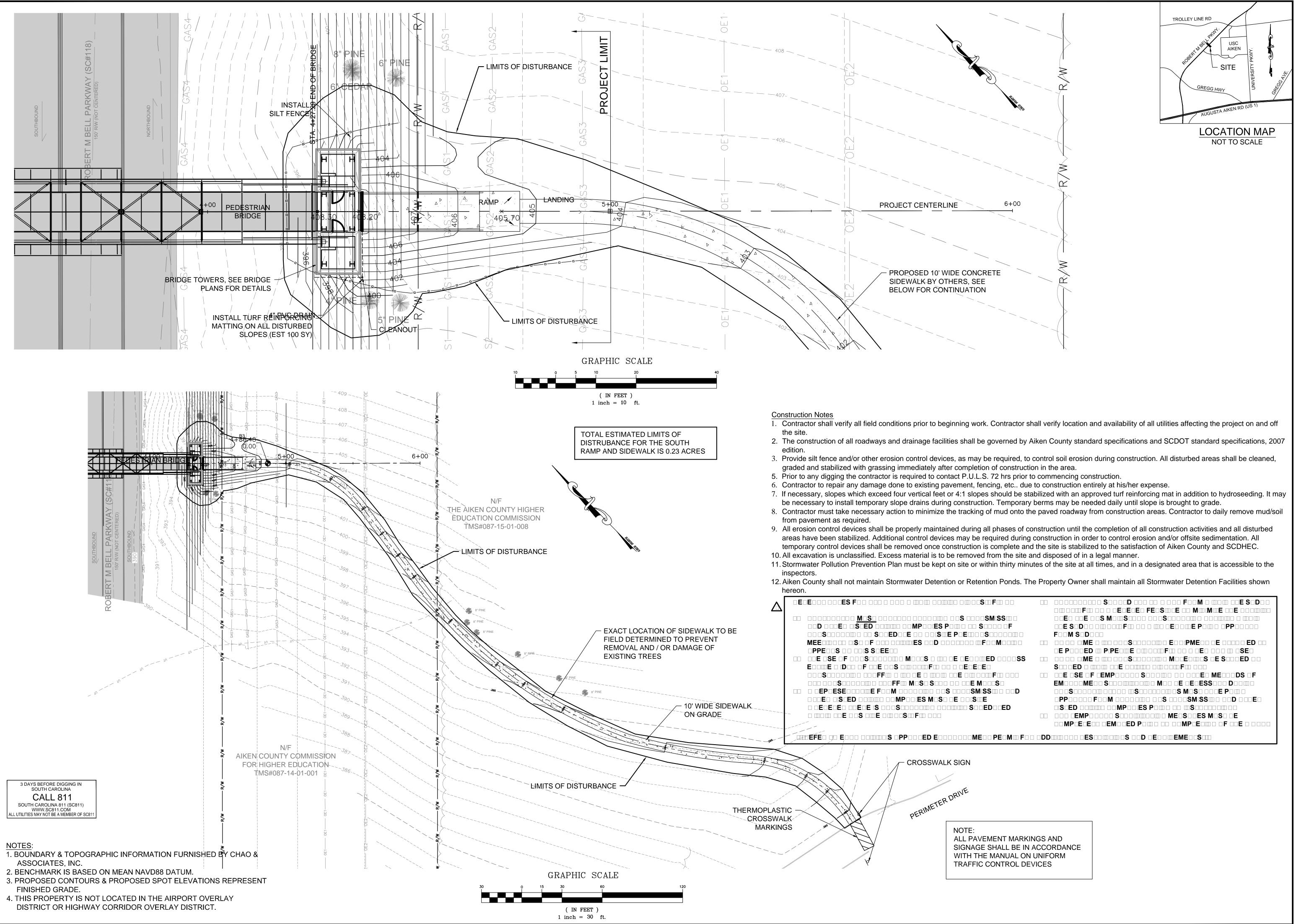
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CA Pedestrian Bridge
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en County, SC

:d: Added GENERAL NOTES FOR ALL WORK WITHIN UTILITY RIGHTS-OF-WAY

Sheet Number

October 9, 2012





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**d Erosion** rian Bridge No. H29-954 South Caroli SC Sediment and USCA Pedesti State Project N University of S Aiken County,

MSE Wall Notes:

Design Methodology:

Permanent structures = 100 years.

addition of the palmetto tree relief.

LRFD Design

Design Life:

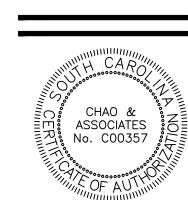
Provide design in accordance with the SCDOT Supplemental

Temporary structures in service for 5 years or longer = 100

Base Bid is MSE wall with no emblem. Alternate # 2 is the

Temporary structures in service for less than 5 year = 5 years.

Technical Specification for Mechanically Stabilized Earth (MSE)



E □ all Ele □ations

CA Pedestrian Bridge

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Versity of South Carolina

South Carolina

iged alternate notes

Colving Project No : 391402B

C5.0 Sheet Number October 9, 2012

Date

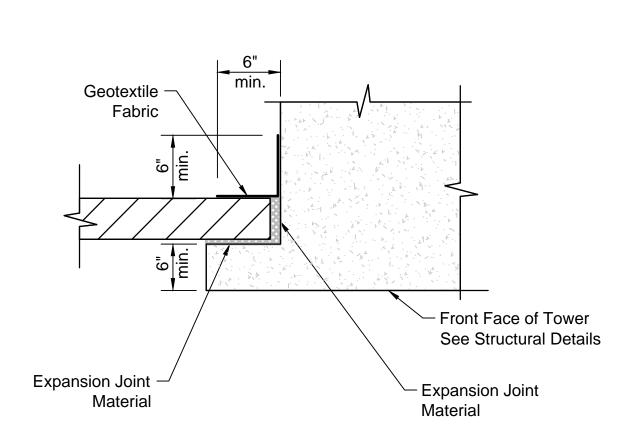
## Elevation View of MSE Retaining Wall (Looking North)

GRAPHIC SCALE

5 0 2.5 5 10 20

( IN FEET )
1 inch = 5 ft.

- 1 Construct 1'-0" x 1'-0" aggregate drain using 6" dia. perforated pipe. Provide aggregate, other than Macadam, that meets the requirements for stone backfill in Supplemental Technical Specification SC-M-713. Wrap geotextile for drainage filtration (see Supplemental Technical Specification SC-M-713) completely around aggregate drain and overlap 1'-0". Design MSE wall drainage system to drain the aggregate drain.
- 2 Extend top two layers of soil reinforcement 5 feet beyond the end of the lower layers of soil reinforcement.
- 3 Slope 2% min. Provide unperforated pipe connecting aggregate drain to drain at wall facing.
- (4) Maximum vertical spacing of soil reinforcement is 36".
- 5 Provide rodent screen manufactured from T304 stainless steel or galvanized steel with a minimum wire diameter of 0.050". Provide rodent screen with minimum of 2 openings per inch and a maximum of 4 openings per inch.
- 6 Limit step height for panel facing to 1/2 of the full panel height. Separate adjacent steps in leveling pad by a minimum of 2 full panels.
- 7 Angle to be determined by the Contractor based on site conditions and the method of construction used. Excavation and/or shoring of retained backfill to permit construction of the MSE wall is considered incidental to the MSE wall construction and is not paid for as a separate item.
- 8 Provide aggregate, other than Macadam, that meets the requirements for stone backfill in the Supplemental Technical Specification SC-M-713.



MSE Wall Interface with Tower Base N.T.S.

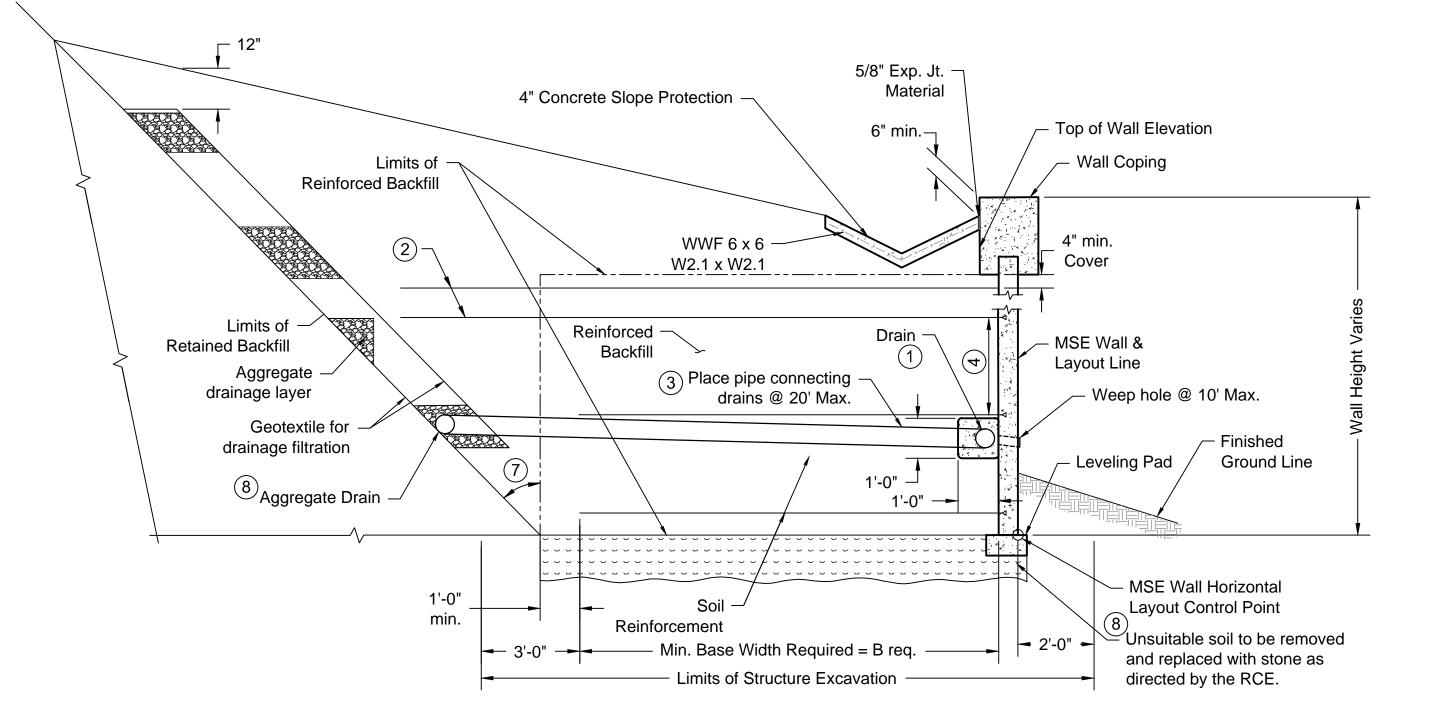
3 DAYS BEFORE DIGGING IN SOUTH CAROLINA

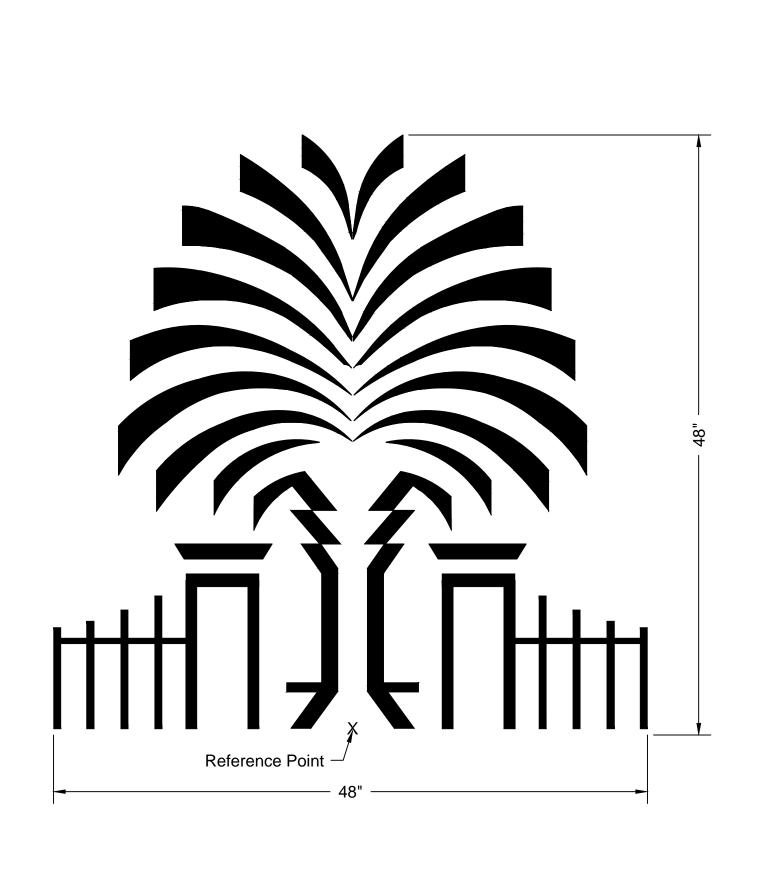
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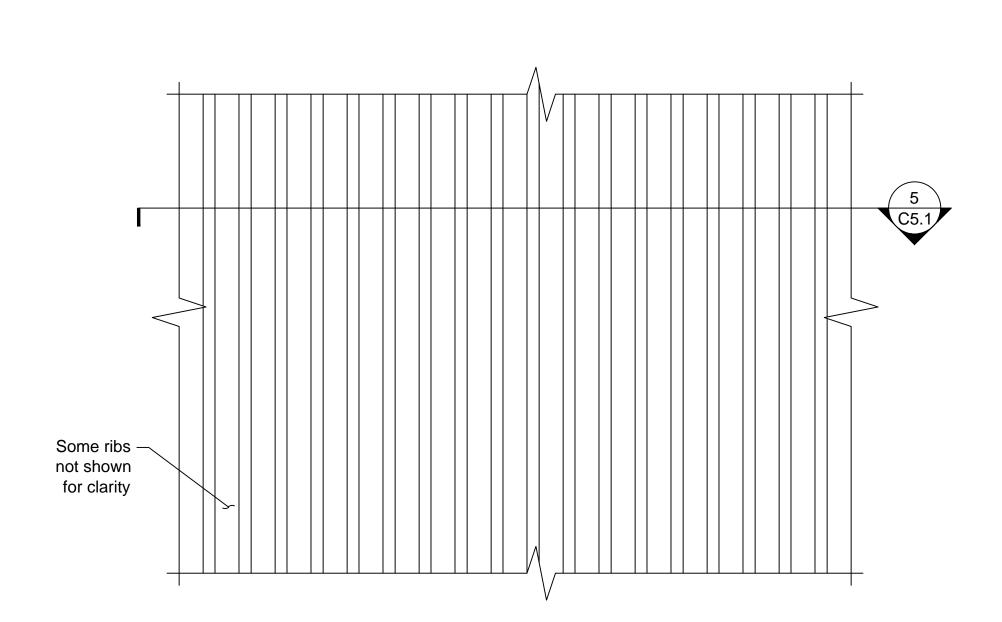




# ALTERNATE # 2

3 - Typical Fracture Ribs

1 - Elevation Palmetto Tree Scale: NTS



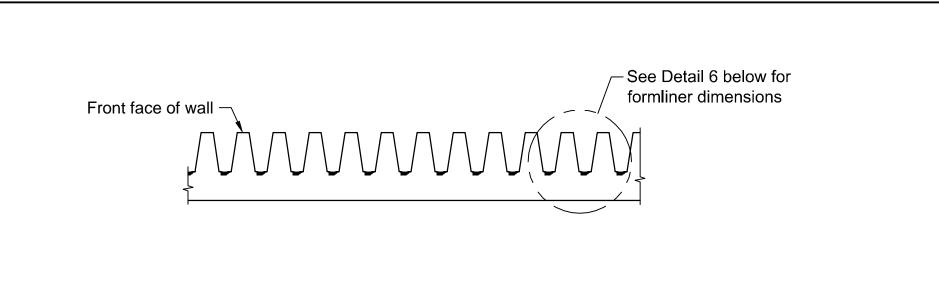
Some ribs not shown for clarity

## ALTERNATE # 2

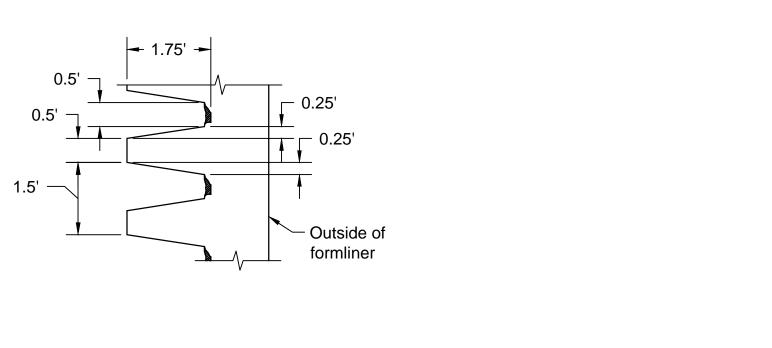
4 - Section

2 - Typical Palmetto Tree Relief

\_ See 6/S3.2 for formliner dimensions Front face of wall  $\lnot$ ( Alternate #3 Only)



5 - Section Scale: NTS



6 - Fractured Rib Concrete Formliner

Scale: NTS

Scale: NTS

Scale: NTS

- 1. Graphic scale should not exceed 75% of the above grade structure height. Install all graphics above finished grade. Apply contrasting textures when using graphics, generally smooth finish and fractured finish.
- 2. When graphics must extend into adjacent panels, a minimum of 25% of the graphics should be contained in each of the adjacent panels.
- 3. Raise or recess graphic below adjacent background as indicated in plans. When graphic is adjacent to formliner texture, graphic is "recessed" below texture surface. When graphic is adjacent to smooth surface, graphic is "recessed" below smooth surface. Raise graphic when graphic, formliner texture and smooth texture are all present and graphic is adjacent to
- 4. Use only formliner systems that are compatible with SCDOT concrete class and construction methods specified for the structure.
- 5. Provide a copy of the formliner manufacturer's installation instructions to resident engineer before fabrication of the structure. Provide copies of material safety data sheets for all materials needed to complete the
- 6. Follow formliner manufacturer's specifications for installation and removal of formliner to produce an architectural finish that appropriately fills the form and is free of voids, pits or other defects.
- 7. Follow manufacturer's finishing procedures to treat seam overlaps that form concealed joints.
- 8. Formliners may be reused within manufacturer's specifications as long as the texture can be achieved without surface defects.
- 9. Color to be selected by Project Architect.
- 10. Provide a copy of the manufacturer's paint installation instructions to the Project Architect before commencing painting. Provide copies of material safety data sheets for all materials needed to complete the application.
- 11. See Standard Specifications for painting applications.
- 12. Contractor's bid to include cost of labor, material and equipment of architectural treatment and painting.

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Chao



October 9, 2012

- 2. Stabilization measures shall be initiated as soon as practicable in portions of the site where construction activities have temporarily or permanently ceased, but in no case more than fourteen (14) days after work has ceased, except as stated below.
  - > Where stabilization by the 14th day is precluded by snow cover or frozen ground conditions
  - stabilization measures must be initiated as soon as practicable.
  - > Where construction activity on a portion of the Site is temporarily ceased, and earth-disturbing activities will be resumed within 14 days, temporary stabilization measures do not have to be initiated on that portion of the Site.
- 3. All sediment and erosion control devices shall be inspected every seven (7) days. If site inspections identify BMPs that are damaged or are not operating effectively, maintenance must be performed as soon as practical or as reasonably possible and before the next storm event whenever practicable.
- 4. Provide silt fence and/or other control devices, as may be required, to control soil erosion during utility construction. All disturbed areas shall be cleaned, graded, and stabilized with grassing immediately after the utility installation. Fill, cover, and temporary seeding at the end of each day are recommended. If water is encountered while trenching, the water should be filtered to remove any sediments before being pumped back into any waters of the State.
- 5. All erosion control devices shall be properly maintained during all phases of construction until the completion of all construction activities and all disturbed areas have been stabilized. Additional control devices may be required during construction in order to control erosion and/or offsite sedimentation. All temporary control devices shall be removed once construction is complete and the site is stabilized.
- 6. The contractor must take necessary action to minimize the tracking of mud onto paved roadway(s) from construction areas and the generation of dust. The contractor shall daily remove mud/soil from pavement, as may be required.
- 7. Residential subdivisions require erosion control features for infrastructure as well as for individual lot construction. Individual property owners shall follow these plans during construction or obtain approval of an individual plan in accordance with S.C Reg. 72-300 et seq. and SCR100000.
- 8. Temporary diversion berms and/or ditches will be provided as needed during construction to protect work areas from upslope runoff and/or to divert sediment-laden water to appropriate traps or stable outlets.
- 9. All waters of the State (WoS), including wetlands, are to be flagged or otherwise clearly marked in the field. A double row of silt fence is to be installed in all areas where a 50-foot buffer can't be maintained between the disturbed area and all WoS. A 10-foot buffer should be maintained between the last row of silt fence and all WoS.
- 10. Litter, construction debris, oils, fuels, and building products with significant potential for impact (such as stockpiles of freshly treated lumber) and construction chemicals that could be exposed to storm water must be prevented from becoming a pollutant source in storm water discharges.

### Seeding Schedule:

Actual types and application rates may vary depending on soil type. Contractor should consult with the local conservation district office for specific recommendations.

The following rates are per 1000 S.F.

MAY 1. - AUG. 31 SEPT. 1 - APR. 30 2 Pounds Annual Rye Grass

1 Pound Browntopmillet 2 Pounds Hulled Bermuda

25 Pounds10-10-10 Fertilizer

75 Pounds Limestone

1 Pound Browntopmillet

\*2 Pounds Bahia Grass

75 Pounds Limestone

**DEEP SANDY SOILS** 2 Pounds Browntopmillet

\*3 Pounds Bahia Grass

25 Pounds 10-10-10 Fertilizer 75 Pounds Limestone

2 Pounds Annual Rye Grass 1 Pound Hulled Bermuda 0.5 Pound Hulled Bermuda 1 Pound Unhulled Bermuda 25 Pounds 10-10-10 Fertilizer \*2 Pounds Bahia Grass 25 Pounds 10-10-10 Fertilizer 75 Pounds Limestone

1 Pound Annual Rye Grass \*\*3 Pounds Fescue Grass

0.5 Pound Hulled Bermuda

75 Pounds Limestone

1.5 Pounds Unhulled Bermuda

25 Pounds 10-10-10 Fertilizer

25 Pounds 10-10-10 Fertilizer 75 Pounds Limestone **DEEP SANDY SOILS** 

75 Pounds Limestone

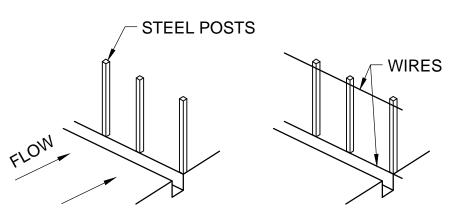
1 Pound Unhulled Bermuda 2 Pounds Rye Grass or 2 lbs Grain Rye \*2 Pounds Bahai Grass 25 Pounds 10-10-10 Fertilizer

\* Bahia grass is a good erosion control grass; however, it produces many seed heads which scatter to other areas. Bahia Grass is hard to mow, but it does not require the care that many other grasses require. Around office buildings and within subdivisions, use 4 to 6 ounces of Centipede Grass on lieu of Bahia Grass or in combination with Bermuda Grass.

\*\* For soils with clay subsoil. Do not plant in sandy soils.

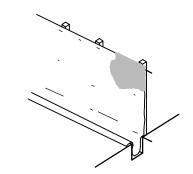
- On all vegetated swales or ditches with side slopes (cut or fill) steeper than 2:1, add 4 to 6 ounces per 1,000 square feet of Weeping Love Grass Seed to any of the above mixtures.

- Swale and ditch bottoms should be double seeded.
- All slopes steeper than 2:1 shall be hydroseeded.
- Growth of Rye Grass in the early spring must be supressed to prevent Rye from choking out perement grass such as Bermuda, Bahia or Fescue.



1. SET POST AT 6' SPACING AND EXCAVATE A 4" X 4" TRENCH UPSLOPE OF THE LINE OF POSTS.

2. ATTACH WIRES TO THE



3. ATTACH THE FILTER FABRIC TO WIRES FABRIC AND EXTEND IT INTO THE TRENCH.

4. BACKFILL AND COMPACT THE EXCAVATED SOIL.

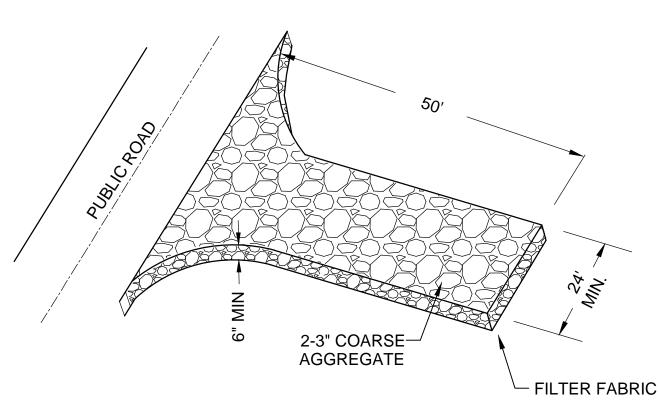
**MAINTENANCE NOTES:** 

1. INSPECT SILT FENCE WEEKLY AND AFTER EACH RAINFALL.

2. REMOVE ACCUMULATED SEDIMENT WHEN IT REACHES 1/2 THE HEIGHT OF THE SILT FENCE.

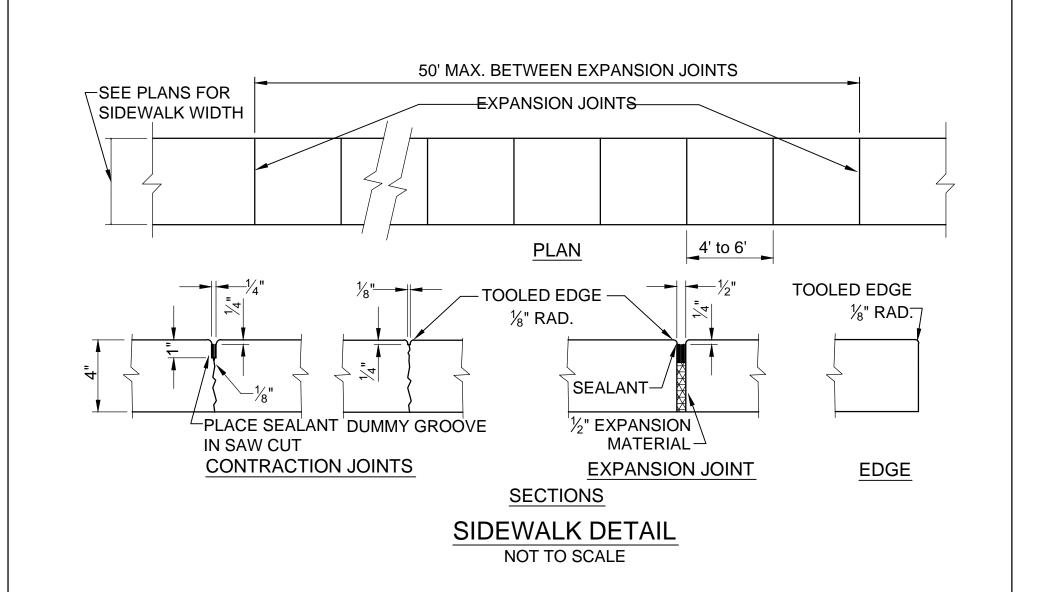
3. REPAIR OR REPLACE FABRIC WHEN TORN OR WEATHERED.

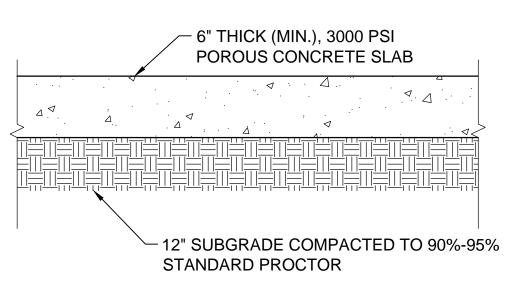
#### SILT FENCE CONSTRUCTION NOT TO SCALE



GRAVEL MAT SHALL BE INSTALLED AT ALL CONSTRUCTION TRAFFIC ACCESS POINTS

#### TEMPORARY GRAVEL CONSTRUCTION ENTRANCE/EXIT NOT TO SCALE





#### NOTE:

#### MANUFACTUREE'S INSTALLATION:

PLACEMENT.

1. FORMWORK MATERIALS ARE PERMITTED TO BE OF WOOD OR STEEL AND SHALL BE THE FULL DEPTH OF THE PAVEMENT.

- 2. PERVIOUS CONCRETE SHALL BE MANUFACTURED AND DELIVERED IN ACCORDANCE WITH ASTM C 94.
- 3. THE BASE SHALL BE IN A DAMP OR SEMI-FLOODED CONDITION AT TIME OF
- 4. PLACING, FINISHING, AND TOOLED JOINTING MUST BE COMPLETED WITHIN 20 MINUTES FROM THE TIME THE PERVIOUS CONCRETE IS DISCHARGED FROM THE TRUCK.
- 5. THE PERVIOUS CONCRETE PAVEMENT SHALL BE COMPACTED TO THE REQUIRED CROSS-SECTION.
- 6. CURING PROCEDURES SHALL BEGIN IMMEDIATELY BUT NO LATER THAN 20 MINUTES FROM THE TIME THE PERVIOUS CONCRETE IS DISCHARGED FROM THE TRUCK.

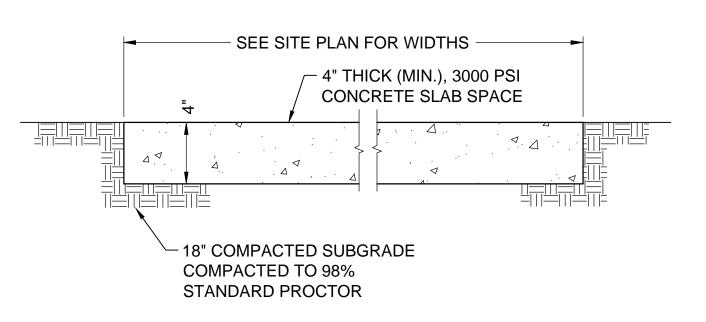
#### MAINTENANCE SPECIFICATIONS

1. THE PAVEMENT SURFACE SHOULD BE VACUUMED BIANNUALLY WITH A

COMMERCIAL CLEANING UNIT

- 2. PLANTED AREAS ADJACENT TO POROUS PAVEMENT SHALL BE WELL MAINTAINED TO PREVENT SOIL WASHOUT ONTO THE PAVEMENT AND SHALL BE INSPECTED ON A SEMI-ANNUAL BASIS.
- 3. ALL TRASH AND OTHER LITTER THAT IS OBSERVED DURING THESE
- INSPECTIONS SHOULD BE REMOVED.
- 4. ALL CONSTRUCTION OR HAZARDOUS MATERIALS CARRIERS SHOULD BE PROHIBITED FROM ENTERING A POROUS PAVEMENT LOT IN ORDER TO PREVENT FROM TRACKING OR SPILLING DIRT ONTO THE PAVEMENT.

#### TYPICAL PERVIOUS CONCRETE PAVEMENT SECTION NOT TO SCALE



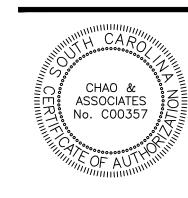
#### TYPICAL CONCRETE SIDEWALK SECTION NOT TO SCALE

**GENERAL NOTE:** CONCRETE TESTING WILL BE PERFORMED BY USC AIKEN 3rd PARTY CONTRACTOR

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Bridge 429-95 η Caroli

**Miscellaneous Details**JSCA Pedestrian Bridge
State Project No. H29-9545-PG
Jniversity of South Carolina
Aiken County, SC

CHAO & ASSOCIATES No. C00357

Miscellaneous
USCA Pedestria
State Project Nc
University of Sol

Drawn: TKS Checked: LEE
Revised:

SECTION A-A

C6.1
Sheet Number
October 9, 2012
Date

SECTION B-B

ACO DRAIN MODEL K100S OR APPROVED EQUAL

TRENCH DRAIN DETAIL

NOT TO SCALE

TOP OF CURB

3 DAYS BEFORE DIGGING IN SOUTH CAROLINA CALL 811

SOUTH CAROLINA 811 (SC811)
WWW.SC811.COM
ALL UTILITIES MAY NOT BE A MEMBER OF SC811

REMOVE PORTION OF EXISTING

INSTALL DEPRESSED CURB

CURB AND GUTTER

RAMP (DEPRESSED CURB)

NOT TO SCALE

#### **PAYMENT**

This project is a lump sum bid including the installation of the piles. Bidder shall include all anticipated costs in his/her bid.

#### COORDINATION OF PLANS, SPECIFICATIONS, AND SPECIAL PROVISIONS

Generally, in case of discrepancy, this General Notes sheet governs over the Standard Specifications but the remainder of the plans govern over notes on this sheet and Special Provisions govern over all. See Subsection 105.4 of the Standard Specifications.

#### REINFORCING STEEL

Fabricate reinforcing bars in accordance with the current C.R.S.I. Manual of Standard Practice except for ties, stirrups, and welded hoops.

Provide all ties and stirrups with 135° hooks that have extensions no less than the larger of ten bar diameters or six inches. This 135° hook requirement does not apply to stirrups extending from prestressed concrete beams.

The fabrication tolerance for out-to-out dimension of welded hoop diameter is ±1/2 inch.

Do not use lap splices in column and shaft reinforcing steel.

#### CONCRETE

Provide the class of concrete as noted in the contract documents. For cast-in-place structural elements, use Class 4000 concrete where the class of concrete is not specified in the contract documents.

When holes are cast in beams to accommodate falsework, fill the holes with a non-shrink structural grout suitable for overhead repairs after falsework is removed.

After erection of the beams and prior to the erection of the deck slab falsework, measure beam cambers. Compare the measured beam cambers to the values shown on the Plans to aid in determining if field adjustments are needed. Submit beam camber measurements and any proposed field adjustments to the RCE for approval. All cost of performing this work is considered incidental to the Contract and no additional compensation is allowed for the performance of this work.

Payment for concrete in slab is based on theoretical plan quantity. No adjustment is made for variation in camber.

Chamfer all exposed edges 3/4" unless otherwise noted.

The minimum acceptable concrete cover for reinforcing steel is 1/2 "less than the plan dimensions when required by reinforcing bar fabrication tolerances.

#### DECK COATING

Deck coating by PSI or approved equal. Surface preparation shall be strictly in accordance with manufacture's requirement. Deck coating shall have 1 layer of Enviro-Grip-MP (Metal primer), 1 layer of P-Tuff Classic (base course), 1 layer Top Shield 5600 (top coat). Reinforcement tape shall also be utilized and embedded in base course layer at crack, control joint, construction joint locations. The thickness of each layer and installation shall comply with manufacture's specifications.

#### GRINDING & TEXTURING CONCRETE DECKS

For bridge stage construction projects, grind and texture the bridge decks as necessary near the stage longitudinal construction joints in order to meet the longitudinal and transverse rideability and rolling straightedge requirements of the Contract.

Prior to casting any closure pour, grinding, or texturing, make profile line surveys (2 to 6 as determined by the RCE) of each stage of the bridge decks. Make one of these profile line surveys for each stage along the edge of the deck adjacent to the closure pour. Compare the surveys within each stage and compare the surveys of each stage to surveys of the adjacent stage to aid in determining the amount of grinding and texturing needed to meet the rideability and rolling straightedge requirements. Submit all grinding and texturing procedures, plotted survey profiles, and proposed grinding depths to the RCE for approval. Maintain a final cover of 2"minimum over the bridge deck reinforcing steel.

Follow the above procedures for all stages of the work. For all surveys performed on the same bridge, use identical stations for survey shots in order to facilitate survey comparisons. All costs for performing, evaluating, and submitting the surveys are considered incidental to the Contract and no additional compensation is allowed for the performance of this work.

#### ALLOWANCE FOR DEAD LOAD **DEFLECTION & SETTLEMENT**

In setting forms for structural steel or prestressed concrete beam spans, apply an allowance to the design finished grade to compensate for computed dead load deflections.

Prior to making deck pours on any stage construction work, and bridge widening projects, consider and make adjustments as necessary for partially loaded beams adjacent to closure pour areas. Verify that any proposed adjustment on partially loaded beams does not create a change in the deck thickness or a reduction in the concrete cover over the reinforcing steel. Welded studs on steel beams and reinforcing steel extending up out of prestressed beams shall meet the requirements for a composite section (extend up into the deck past the bottom mat of reinforcing steel) regardless of any adjustments.

In setting falsework for reinforced concrete spans, make an allowance for the deflection of the falsework, for any settlement of the falsework, for the instantaneous dead load deflection of the span, and for the long-time dead load deflection of the span such that on removal of the falsework the top of the structure shall conform to theoretical finished grade plus the allowance for long-time deflection.

For instantaneous and long-time dead load deflection, use a camber of 1/8" for concrete flat slab spans 22 feet in length, 3/16" for concrete flat slab spans 30 feet in length, and 3/8" for concrete flat slab spans 40 feet in length, unless otherwise directed by the RCE. Adjust these cambers as necessary to allow for falsework deflection, falsework settlement, and vertical curve ordinates.

#### DRIVEN PILE FOUNDATIONS

Where piles occur in fill, place fill before driving piles.

#### STRUCTURAL AND MISCELLANEOUS STEEL

All structural and miscellaneous steel shall conform to the Thirteenth Edition of the AISC "Specification for the Design, Fabrication & Erection of Structural steel for Buildings" and all its supplements, and to the AISC "Code of Standard Practice for Steel Buildings and Bridges".

All structural steel shall conform to ASTM A-36, FY=36,000 PSI unless otherwise

Steel W-Shapes shall conform to ASTM A992, FY=50,000 PSI. All rectangular or square steel HSS-Shapes shall conform to ASTM A500 grade B, FY=46,000 PSI. All round steel HSS-Shapes shall conform to ATSM A500 grade B. FY=42,000 PSI.

All steel pipes shall conform to A-53 grade B, FY=35,000 PSI. All welded connections shall be done with E70XX electrodes with 1/4" min. material, unless otherwise noted.

All bolts shall be A325 bolts, unless otherwise noted. Fabrication and assembly of bolted connections shall comply with applicable sections of AISC "Specification for Structural Joints using ASTM A325 or A490

No openings in beams shall be permitted without the written permission of the

The use of a gas-cutting torch in the field for cutting holes or for correcting fabrication errors will not be permitted on structural framing members except w/ the written approval of the Engineer for each specification.

An independent inspection agency shall be employed by the contractor and approved by the engineer to inspect the structural steel in the field and verify that it conforms to the requirements of the contract documents.

All anchor bolts shall be ASTM F1554 Grade 36, unless noted otherwise.

All steel plates in moment connection shall be ASTM A572 Grade 50. All bolts in moment connection shall be fully tensioned A325-X bearing bolts.

All moment connected members shall be fabricated, erected, inspected and approved in compliance with FEMA-353, "Recommended Specifications and Quality Assurance Guidelines for Steel Moment Frame Construction for Seismic Applications."

Generally, holes for 3/4" dia. bolts shall be 13/16" dia. However, oversize holes, 3/16" larger than bolt dia., may be used in diaphragms and/or crossframes and their connection plates provided hardened washers are installed over oversize holes in the outer ply of the material gripped.
Hardened washers are required under DTIs on oversized holes. In every case install a hardened washer under the element turned for each bolt of a bolted connection. Indicate on the Shop Plans which holes are to be oversize and where hardened washer's are required. No additional payment is made for the costs associated with the use of oversize holes and furnishing additional hardened washers as necessary.

#### PAINT FOR STRUCTURAL STEEL

Paint structural steel in accordance with Section 710 of the Standard Specifications.

# BEARING AND ANCHOR BOLT ASSEMBLIES

Galvanize bearing and anchor bolt assemblies in accordance with AASHTO M 111, AASHTO M 232, or ASTM F 2329 as applicable

After the required field welding of galvanized bearing assemblies, field repair the weld areas and/or damaged areas of the galvanized coating in accordance with

#### ORIENTATION IN RELATION TO STATIONING

Left and right sides, where referred to in these plans, are in relation to direction of stationing.

#### **SPECIFICATIONS**

AASHTO 2010 LRFD Bridge Design Specifications, 5th Edition, with Interim Revisions through 2010.

additions and revisions as stated in the Standard Specifications.

AASHTO LRFD Guide Specifications for the Design of Pedestrian Bridges ANSI/AASHTO/AWS D1.5 Bridge Welding Code (Latest Edition) with

#### DESIGN DATA

1: Dead Loads: The weight of all materials of construction 2: Uniform live loads: Pedestrian: 90 psf 3: Truck Load: H-10

Seismic design is in accordance with the 2008 SCDOT "Seismic Design Specifications for Highway Bridges", Version 2.0 with the following

Seismic Design Category: B Analysis method: No detailed analysis Operational Classification: II Site Class: D **Design Acceleration Coefficients:** PGA (FFE): 0.15g Sds (FFE): 0.26g (FFE): 0.13g Sd1 PGA (SEE): 0.33g (SEE): 0.60g (SEE): 0.33g

Values determined from the Three-Point Method

FINAL FINISH OF EXPOSED CONCRETE SURFACES Apply the final surface finish on the bridge(s) only to the following checked and designated bridge areas:

A) Entire surface of all barrier rails, parapet walls, approach slab curbs, concrete utility supports, and wing walls; outside vertical edge of bridge deck slabs and sidewalks.

B) Outside face of exterior prestressed girders

C) Entire surface of designated substructure units, except top of bent caps and piers.

> All Units Designated Units:

X D) No final surface finish required.

Where notes specify "Department", this refers to the project owner being the University of South Carolina Aiken.

Where notes specify "RCE", this refers to the project Engineer being Chao & Associates, Inc.

Associates

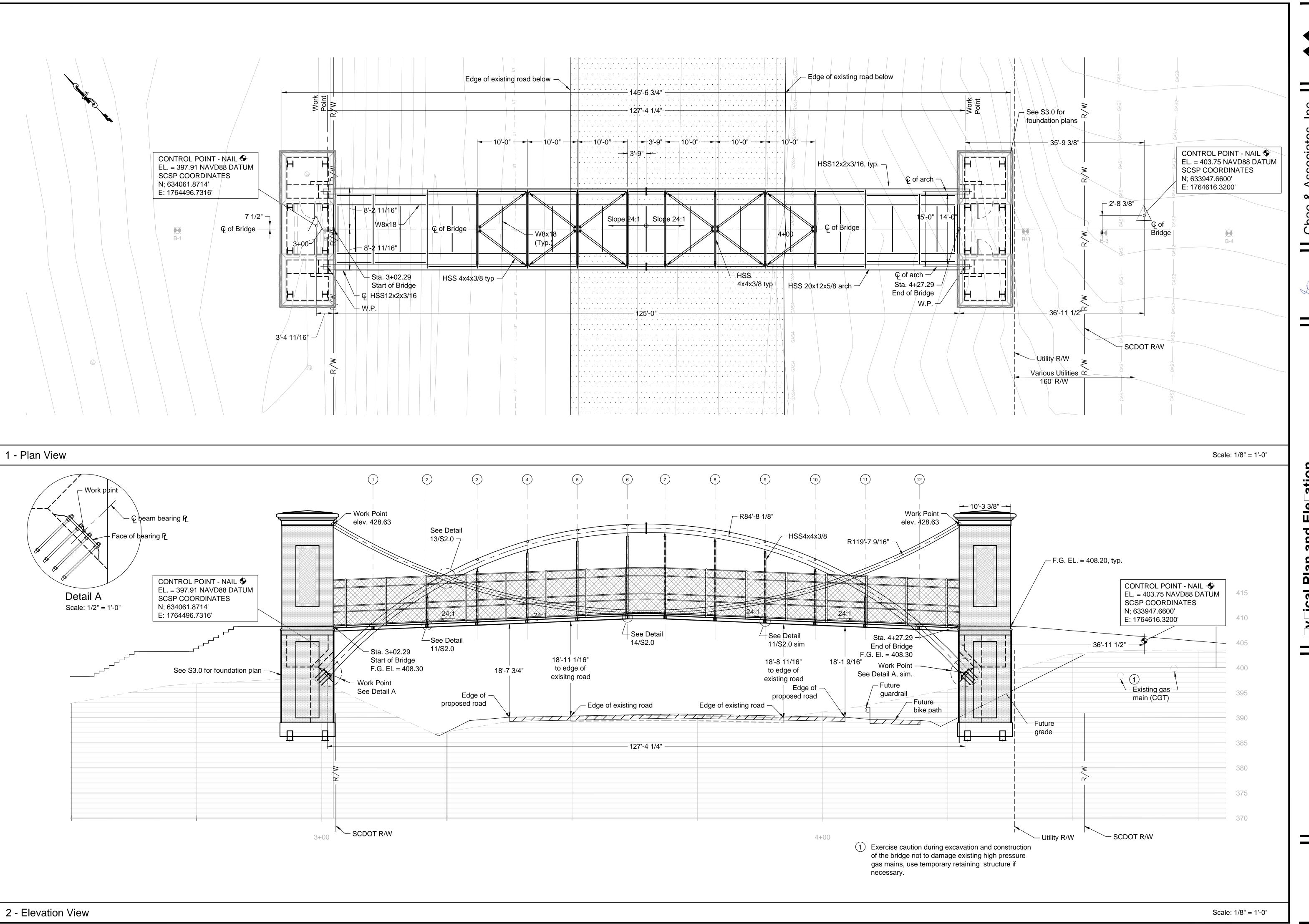
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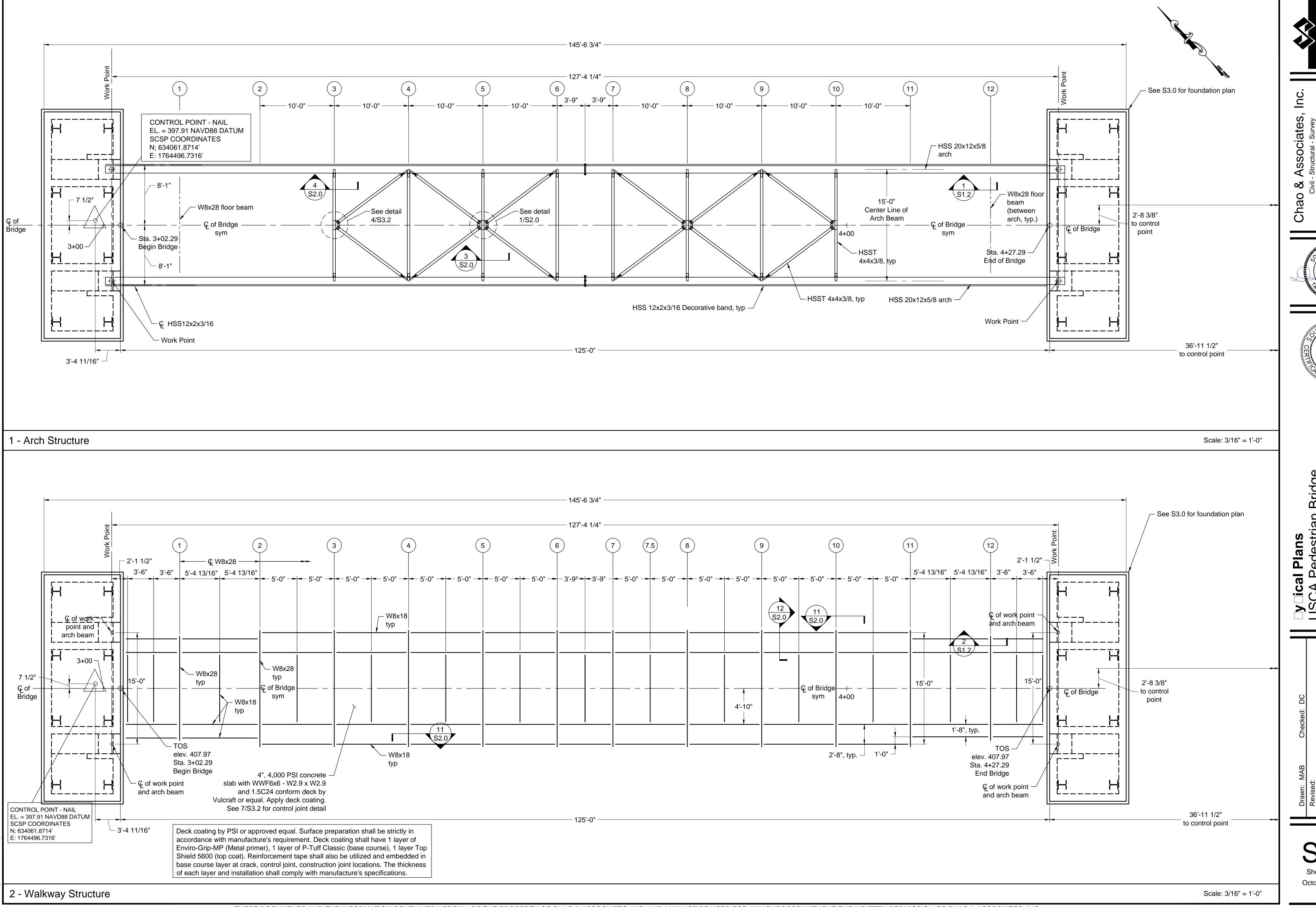


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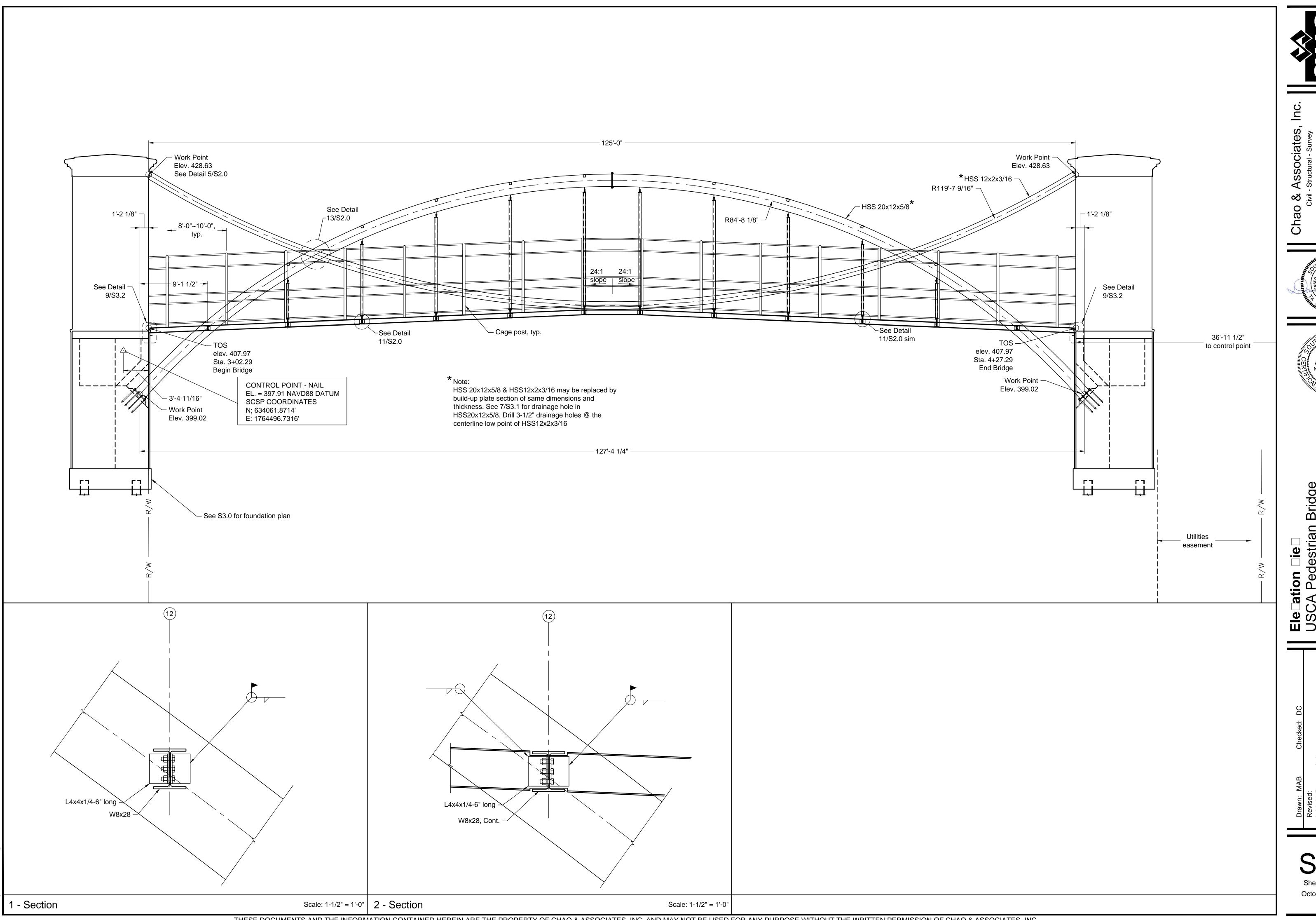
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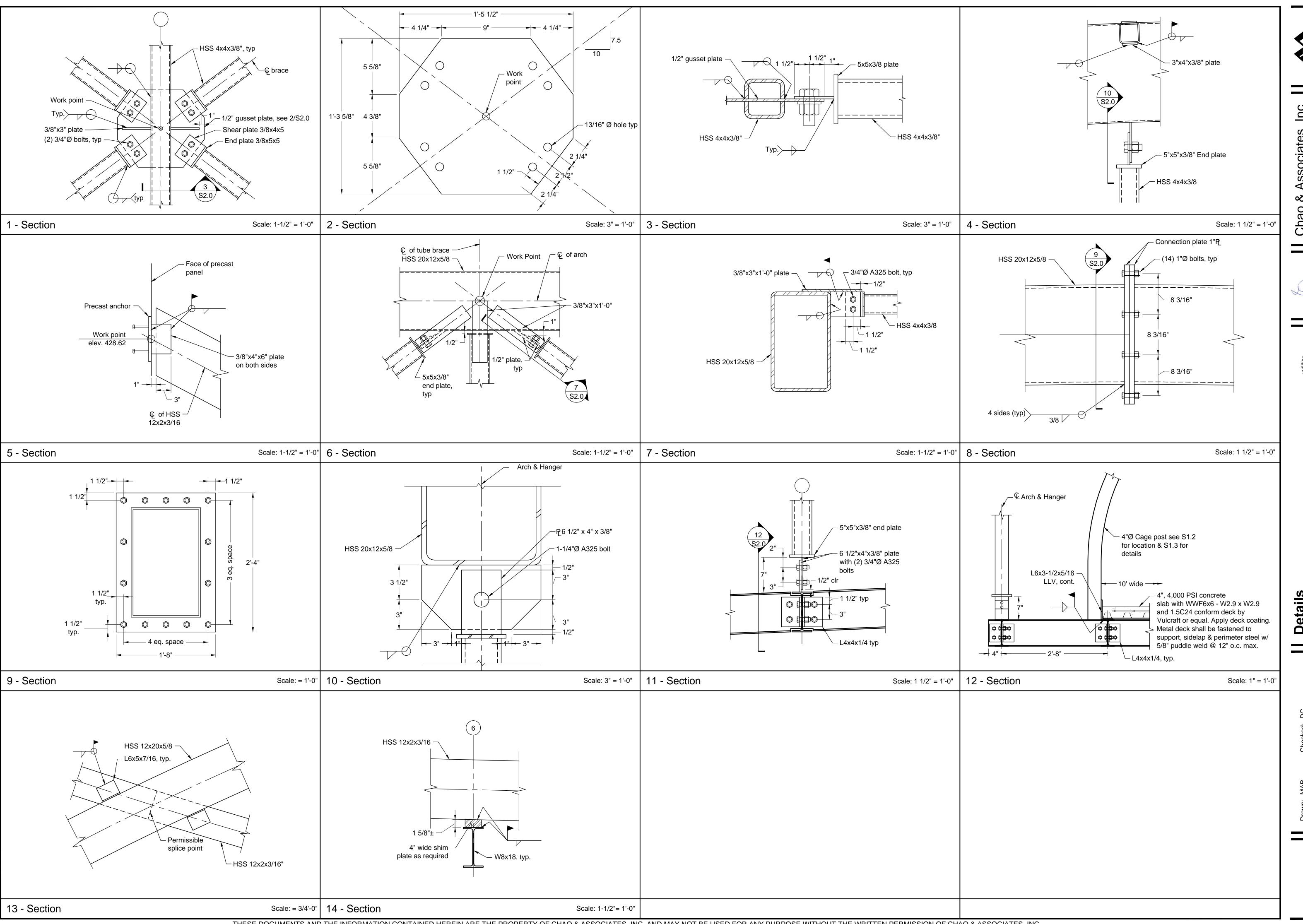
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State Project No. H29-9545-P
University of South Carolina
Aiken County, SC

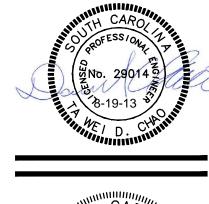
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**Details**USCA Pedestrian Bridge
State Project No. H29-954
University of South Carolin
Aiken County, SC

October 9, 2012 Date

The installation of the driven piles shall be conducted in accordance with Section 7II Driven Pile Foundations section of the SCDOT Standard Specifications for Highway Construction, 2007 edition.

Method of controlling Installation of piles and verifying their capacity: Capacity will be verified by Pile Driving Analyzer (PDA) and CAPWAP analysis on Index piles. A pile Installation chart developed from the analysis will be used to verify the capacity of production piles.

The following parameters should be used for performing drivabillity analysis for the

Pile Bearing (one pile)			
	End Bents		
Factored Design Load	80 tons		
Geotechnical Resistance Factor	0.6500		
Nominal Resistance	123 tons		
Est. Loss Due to Scour	0 tons		
Est. Loss Due to Downdrag	0 tons <sup>(1)</sup>		
Required Driving Resistance	123 tons		
Estimated Pile Tip Elevation (ft, MSL)	349 ft		
Minimum Pile Tip Elevation for Lateral Stability (ft, MSL)	371 ft <sup>(2)</sup>		
(1) Consolidation downdrag (2) Minimum that must be achieved			

Perform pile driving analyzer (PDA) testing on the index piles. Index piles should be lengthened an additional 5 feet in order to accommodate the PDA testing. Drive index pile to grade or practical refusal, whichever occurs first. If a CAPWAP analysis determines that capacity has not been achieved, restrike the index pile. Perform PDA testing during the restrike. Contact the bridge construction office to determine the time between initial driving and restrike.

Each pile is to be installed in one continuous operation. Include details of any anticipated temporary driving discontinuities including anticipated time intervals during the pile installation plan.

A pile hammer having the rated energy indicated is considered suitable for driven pile installation. However final hammer acceptance is based on a wave equation analysis that accurately reflects the contractor's proposed driving system

This project is a lump sum bid including the installation of the piles. Bidder shall include all anticipated costs in his/her bid.

Drivabillity Analysis					
	End Bents				
Pile Type	Hp 14 x 73 Steel Pile				
Skin Quake	0.10				
Toe Quake	0.10				
Skin Damping	0.05				
Toe Damping	0.15				
% Skin Friction	N/A				
% End Bearing	N/A				
Distribution Shape No.	Variable Resistance Distribution (1)				
Bearing Graph	Constant End Bearing (2)				
Pile Penetration	100%				
Hammer Rated Energy Range	20 to 40 ft-kips (3)				

#### Notes

- Variable Unit Skin Distribution: at 5 feet depth = 0; at 27 feet depth = 1.9 ksf; and at 40 feet depth = 2.4 ksf. Variable Unit toe Resistance: at 5 feet depth = 0; at 23 feet depth = 22.0 ksf; at 40 feet depth = 22.0 ksf.
   Constant End Bearing = 22 kips
- Hammer rated energy is based on GRLWEAP maximum rated energy database for diesel hammers

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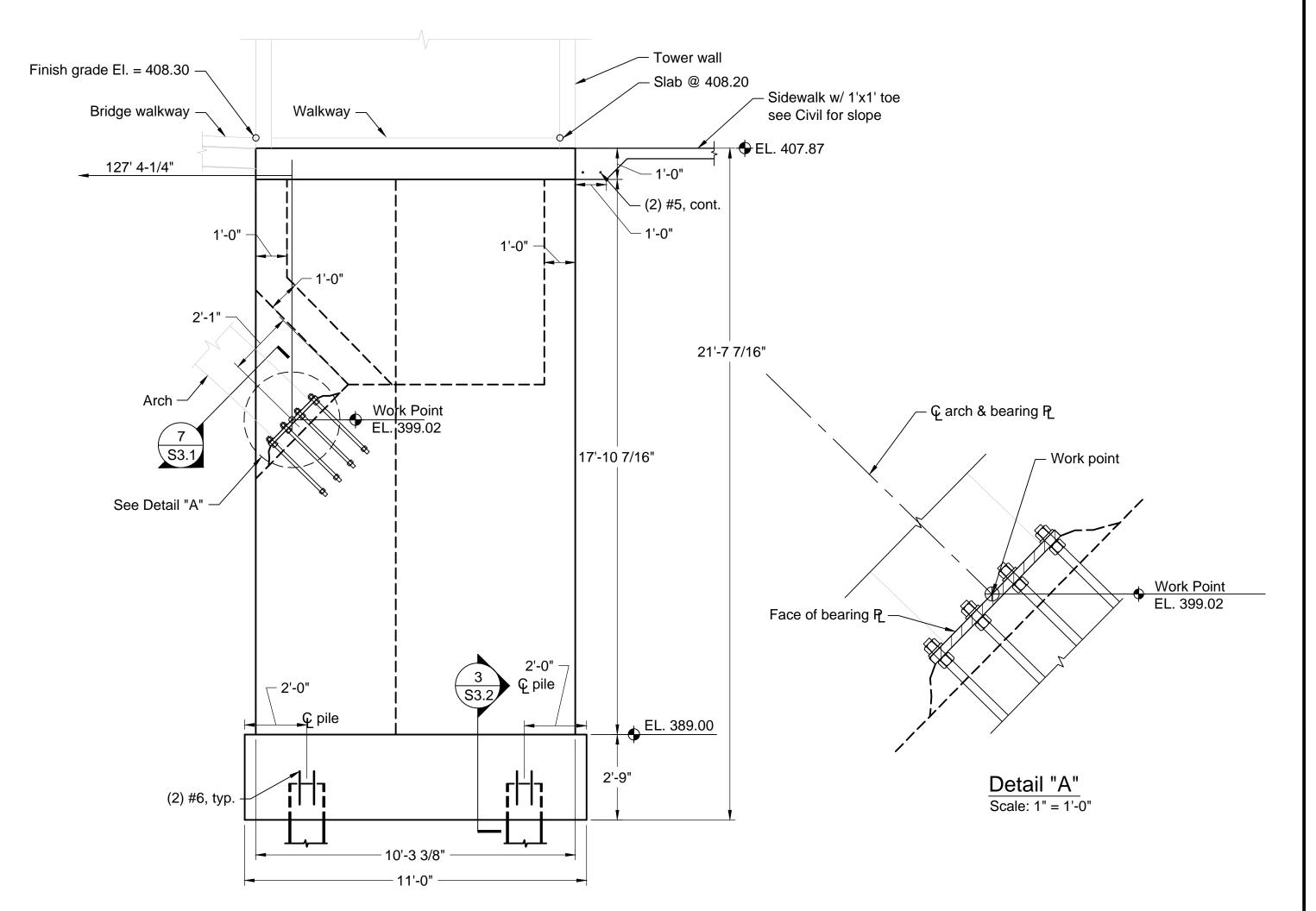
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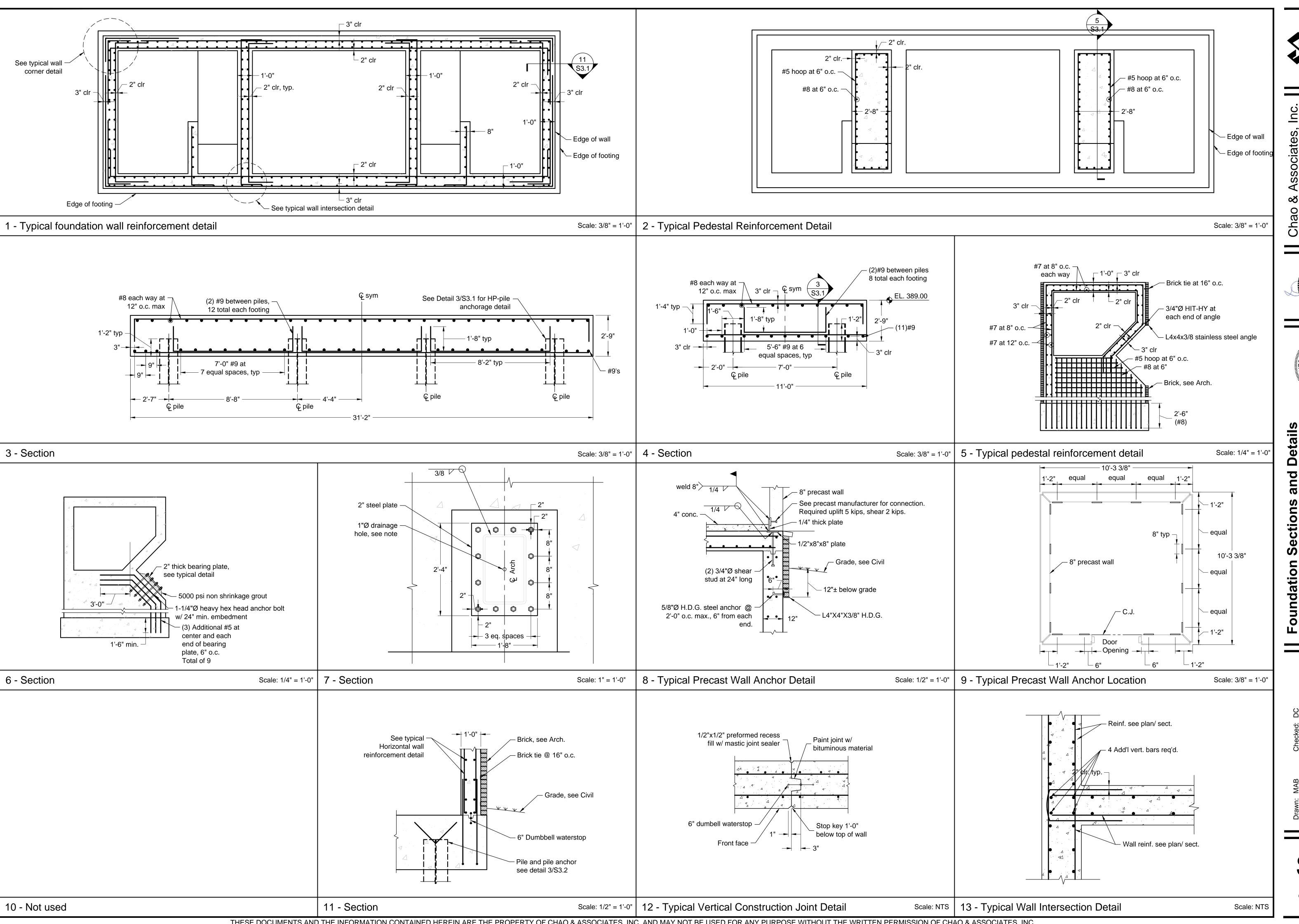
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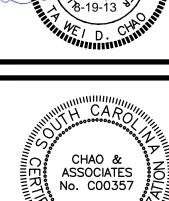
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	30'-5 3/8"	-
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2 \$3.1	6'-2" 2'-8" 6'-4 11/16" 6'-4 11/16" typ.	1'-0"
	3'-0 7/16"	
	8'-7"	
S3.1	G Arch	EL. 389.00
	1'-2", typ.    HP 14x73, typ 8 places    2'-7"    4'-4"    4'-4"    2'-7"	See 3/S3.2 f pile anchora detail, typ.

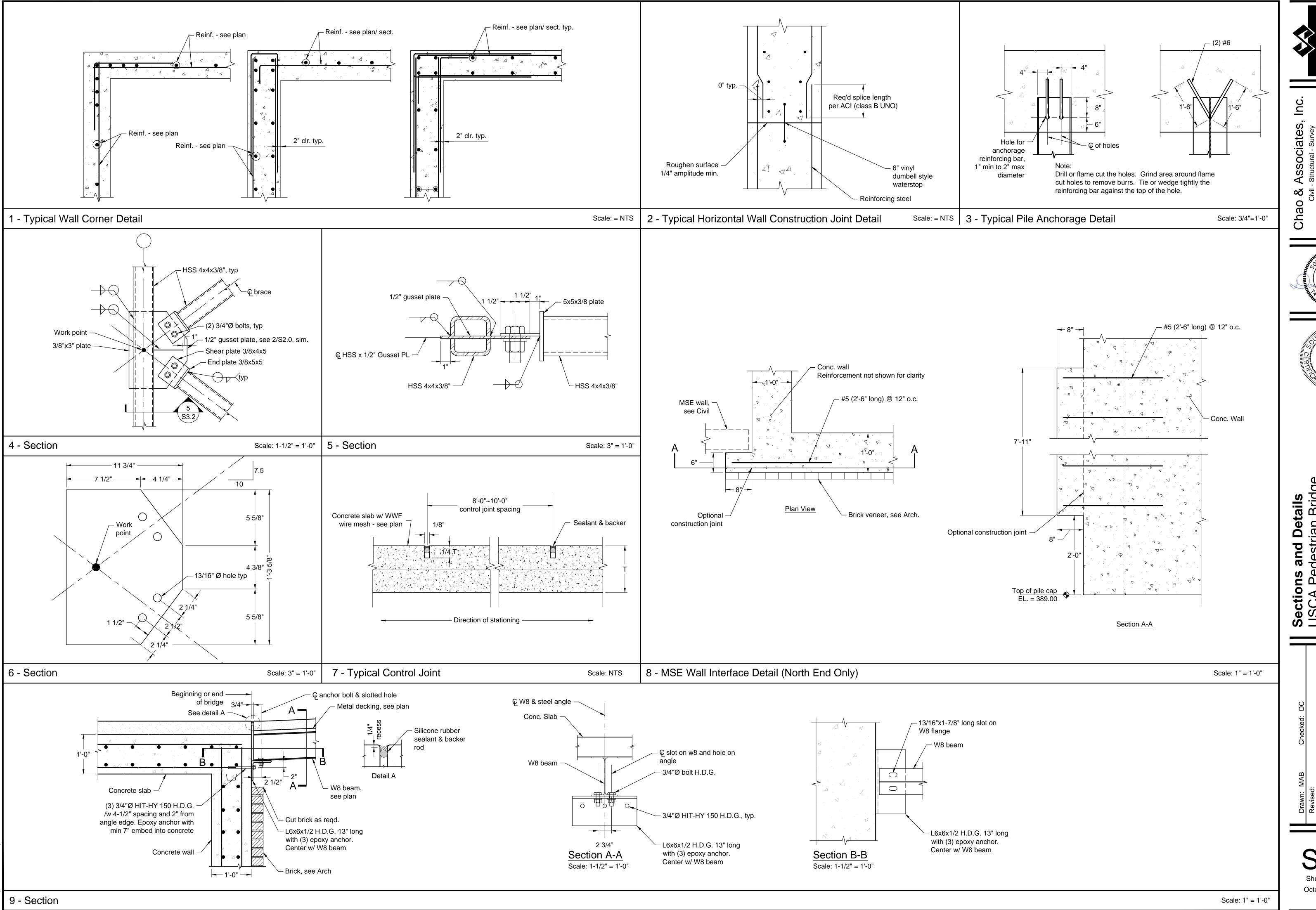




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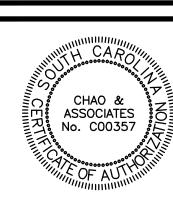
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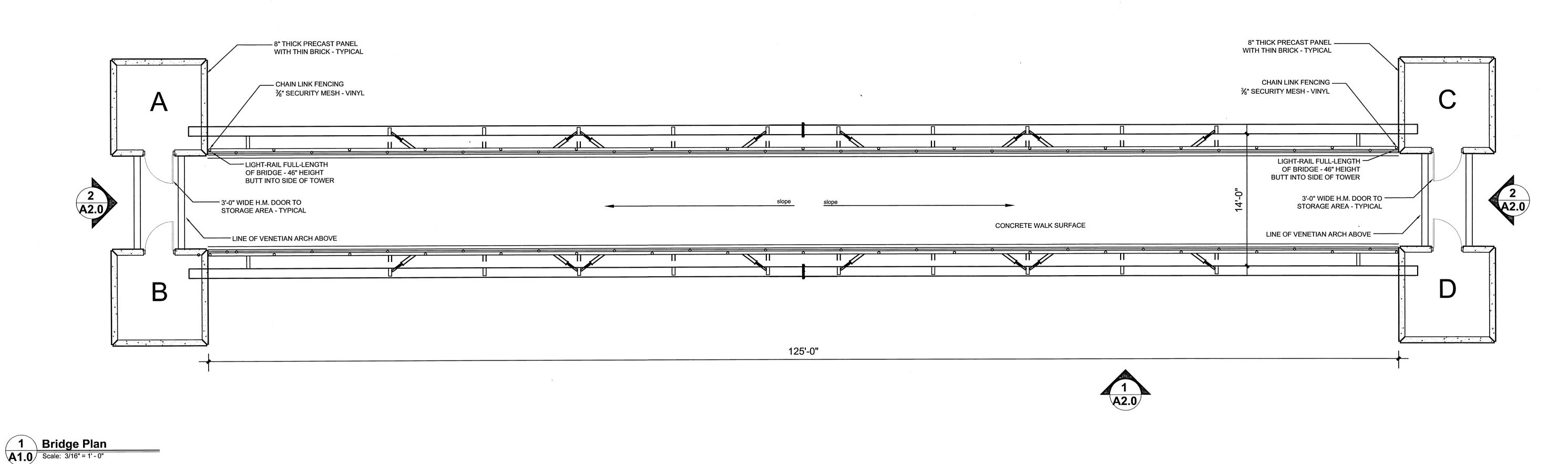
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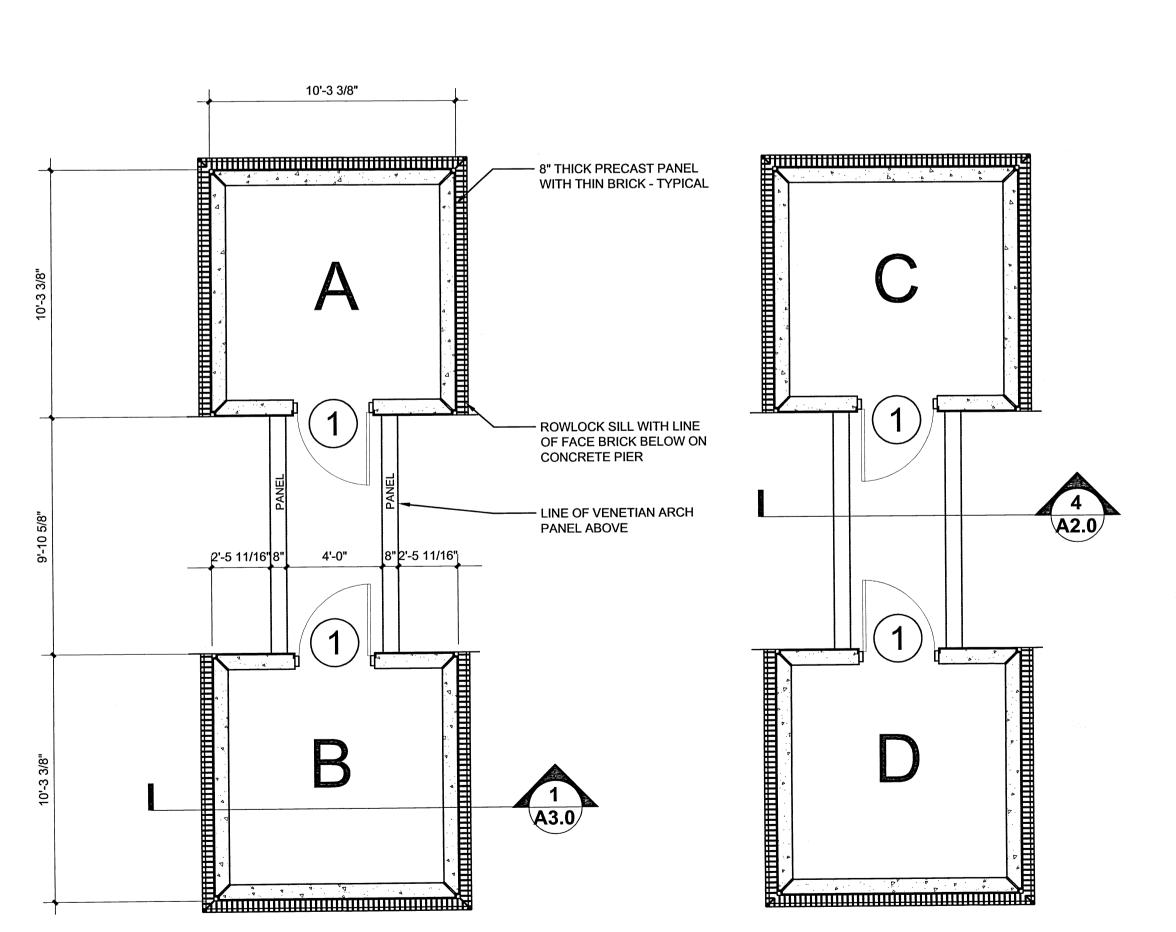
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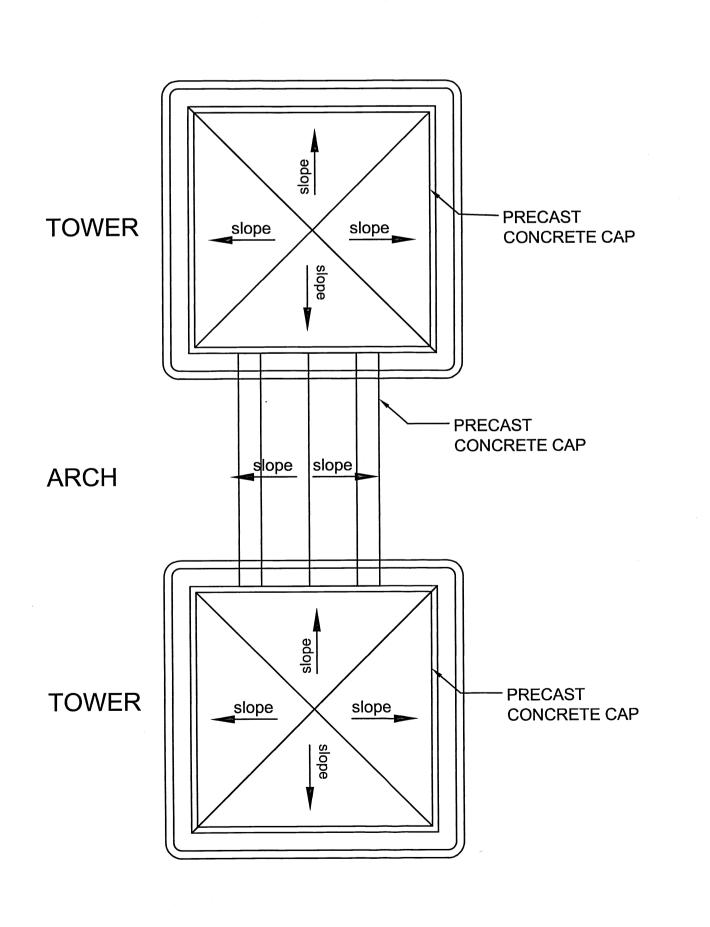
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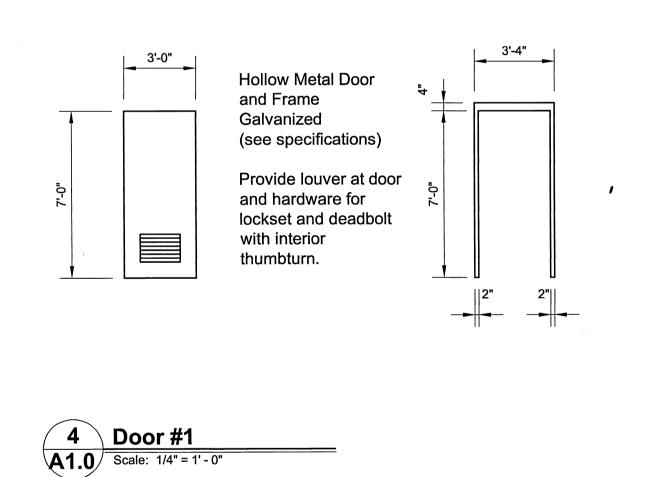
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2 Tower Plans A1.0 Scale: 1/4" = 1' - 0"





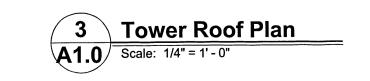
SECURITY CHAIN LINK FENCE FABRIC

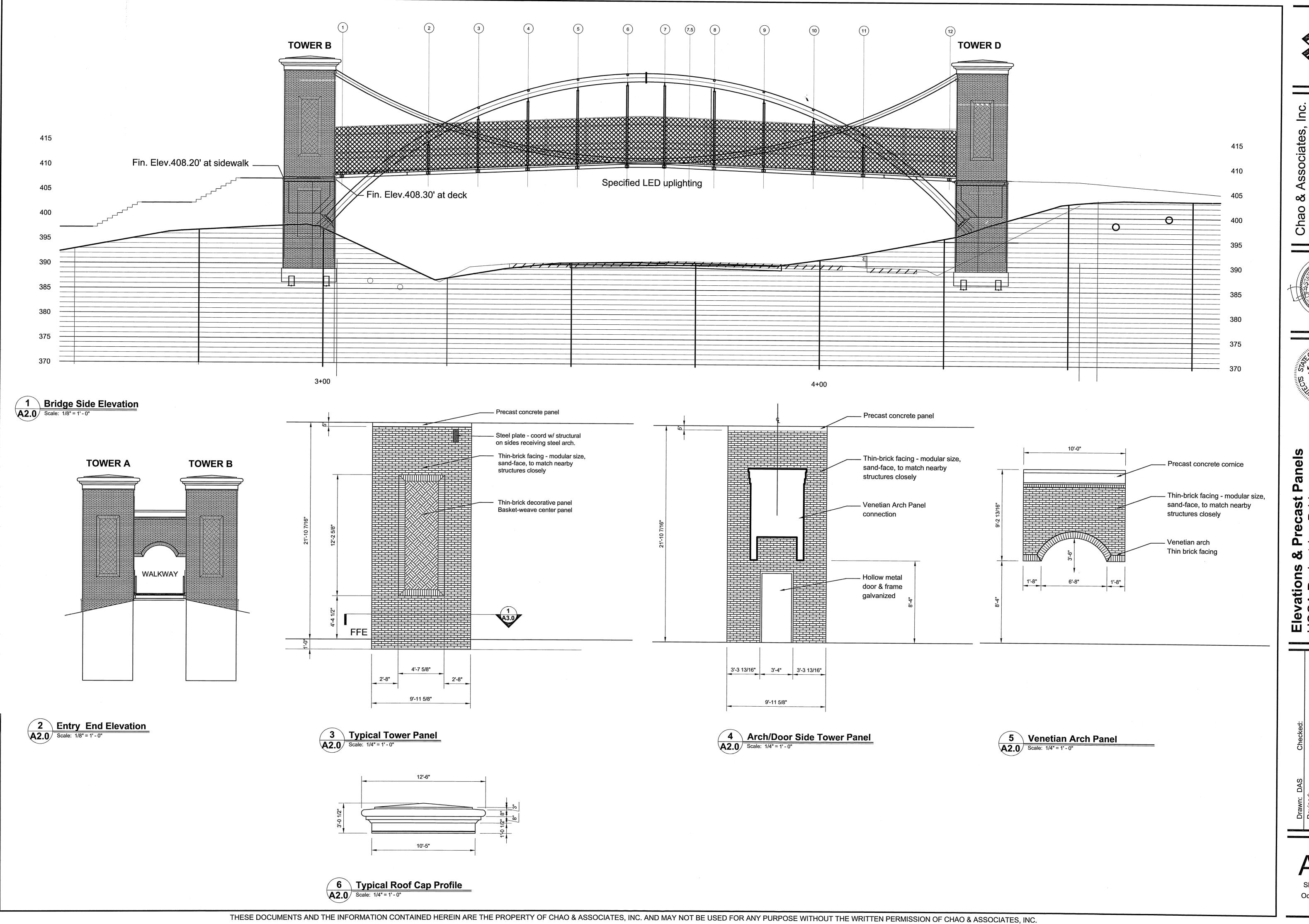
Engineering design of chain-link fence by Contractor. Refer to general specifications for chain link fence.

Materials and workmanship shall carry a 15 year warranty.

Fence fabric: Steel wire mesh sized  $\frac{1}{2}$ " of 9 ga. wire with polymer coating, black color or as selected from manufacturer's standard colors.

Posts and Rails: Heavy industrial strength; round shape, as noted on structural drawings (sheet S1.3). Polymer coated to match fabric mesh.

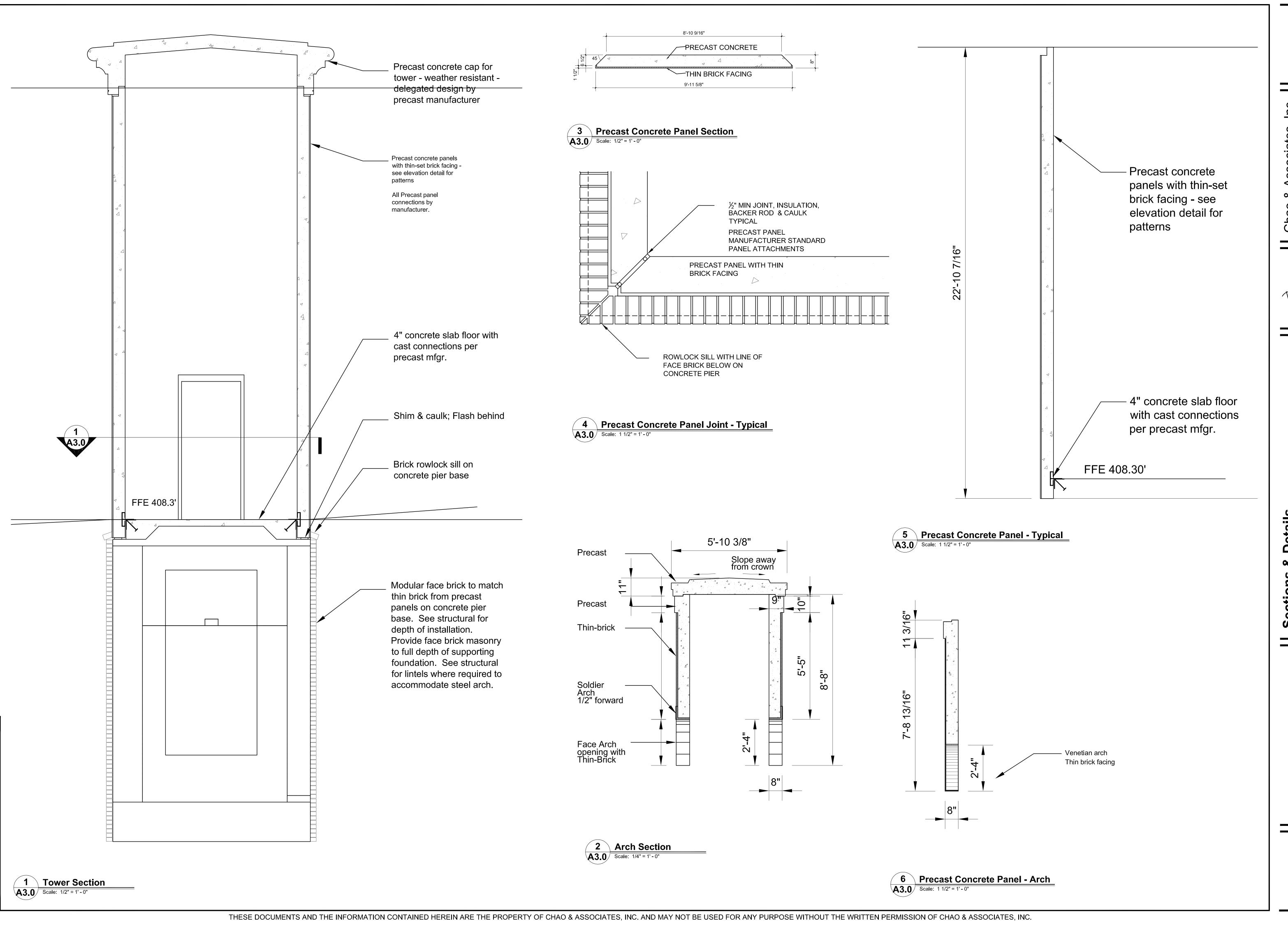








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State Project No. H29-9545-F
University of South Carolina
Aiken County, SC





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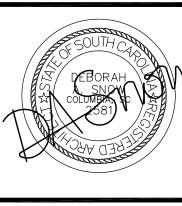
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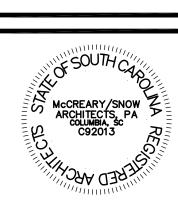
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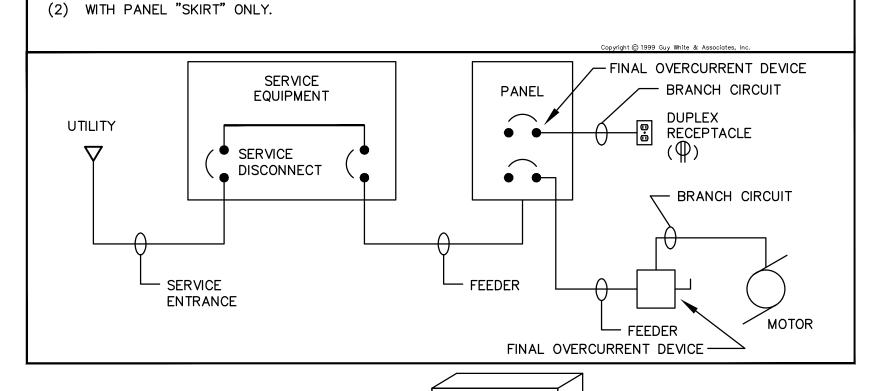
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Sections & Details
USCA Pedestrian Bridge
State Project No. H29-9545-PC
University of South Carolina
Aiken County, SC

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#### ABBREVIATIONS

THE FOLLOWING STANDARD ABBREVIATIONS ARE USED IN THESE PLANS AND SPECIFICATIONS. CONTRACTOR IS CAUTIONED THAT ALL ABBREVIATIONS LISTED MAY NOT BE USED: CONSULT PLANS AND SPECIFICATIONS FOR ABBREVIATIONS APPLICABLE TO THIS PROJECT.

WEATHERPROOF FLEXIBLE CONDUIT

ABOVE FINISHED FLOOR B.F.F. BELOW FINISHED FLOOR A.F.G. ABOVE FINISHED GRADE B.F.G. BELOW FINISHED GRADE U.N.O. UNLESS NOTED OTHERWISE CKT. CIRCUIT CONDUIT E.C. EMPTY CONDUIT FLX. FLEXIBLE CONDUIT

WFLX

#### BRANCH CIRCUIT WIRING -HASHMARK CODE Copyright © 1999 Guy White & Associates, Inc. NO SCALE

BRANCH CIRCUITS SHOWN ON THESE DRAWINGS MAY INCLUDE HASHMARKS WHICH INDICATE THE NUMBER OF WIRES TO BE PROVIDED IN A CONDUIT RUN BETWEEN OUTLETS OR JUNCTION BOXES. WIRE SIZES SHALL BE AS TABULATED IN PANELBOARD SCHEDULES UNLESS OTHERWISE INDICATED ON PLAN. SEE SYMBOL SCHEDULE FOR CONDUIT ROUTING NOTATION. HASHMARK CODE IS AS FOLLOWS:

EACH PHASE AND NEUTRAL WIRE IN A CONDUIT RUN IS REPRESENTED BY A HASHMARK FOR EXAMPLE -

TWO WIRES (NO HASHMARKS) — /// ► THREE WIRES (3 HASHMARKS) — ///// ➤ FIVE WIRES (5 HASHMARKS) . . . AND SO FORTH.

NOTE: GROUND WIRES ARE NOT GENERALLY SHOWN. EXAMINE SPECIFICATIONS AND GENERAL NOTES TO DETERMINE REQUIREMENTS FOR GROUND WIRES AND WHERE SPECIFIED, PROVIDE IN ADDITION TO THE NUMBER OF WIRES INDICATED BY HASHMARK

REAM CONDUIT, BUTT

INTO FITTING BODY &

SECURE NUTS WRENCH

\_\_\_\_

COMPRESSION TYPE CONDUIT

FITTING

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NO SCALE

EMPTY CONDUITS ARE NOTED BY "EC" WITH TRADE SIZE.

# 1. DO NOT SCALE DRAWINGS UNLESS DIMENSIONS ARE SHOWN. LOCATE OUTLETS AND

- 2. MINIMUM SIZE CONDUCTOR FOR POWER SHALL BE NO. 12 AWG.
- 3. ALL FUSES SHALL BE DUAL-ELEMENT TYPE, "FUSETRON" BY BUSSMAN, OR "ECON" BY ECONOMY.
- 4. BRANCH CIRCUIT SIZES ARE AWG 12-1/2"C. UNLESS OTHERWISE NOTED IN PANELBOARD SCHEDULES.
- 5. ALL BRANCH CIRCUIT LOADS SHALL BE BALANCED ACROSS PANELBOARD BUSSES TO OBTAIN MINIMUM NEUTRAL CURRENT.
- 6. ALL FLEXIBLE CONDUIT SHALL CONTAIN A GREEN WIRE BONDED TO RIGID RACEWAY, BOX OR FIXTURE AT EACH END OF FLEX. SIZE GROUND WIRE PER N.E.C. TABLE
- 7. PROVIDE PULL CORD IN ALL EMPTY RACEWAYS.

- STEEL OR MALLEABLE IRON COMPRESSION E.M.T.

FITTING RAIN TIGHT & CONCRETE TIGHT. EFCOR

#761 FOR 3/4" E.M.T. (OTHER FITTINGS EFCOR:

CONNECTOR). EQUALS BY REGAL, T&B, STEEL

#771B 90° CONNECTOR; #751B STRAIGHT

- 8. DO NOT MOUNT FLUSH JUNCTION BOXES BACK TO BACK, STAGGER JUNCTION BOXES TO REDUCE SOUND TRANSMISSION BETWEEN ROOMS.
- 9. CONTRACTOR SHALL MINIMIZE REMOVAL OF STRUCTURAL STEEL FIREPROOFING FOR INSTALLATION OF CONDUIT AND EQUIPMENT HANGERS. OBTAIN APPROVAL OF GENERAL CONTRACTOR PRIOR TO REMOVAL.
- 10 COORDINATE WITH OTHER TRADES TO CONCEAL ELECTRICAL WORK AND PROVIDE OUTLETS IN CORRECT LOCATIONS FOR EACH PIECE OF MECHANICAL OR ELECTRICAL EQUIPMENT CONNECTED.
- 11. CONCEAL OUTLETS FOR ALL EQUIPMENT IN FINISHED AREAS. OBTAIN ROUGHING DIAGRAMS FOR ALL EQUIPMENT AND INSTALL ELECTRICAL WORK ACCORDING TO DIAGRAMS.
- 12. MOUNT BRACKET TYPE LIGHTING FIXTURES AT HEIGHTS SHOWN OR SCHEDULED ON DRAWINGS OR AS DIRECTED ON JOB BY ARCHITECT, U.N.O.

#### ELECTRICAL SYMBOLS

CONDUIT STUB PHOTOCELL, 1800VA U.N.O., AIM NORTH. TIME SWITCH

BRANCH CIRCUIT RACEWAY - CONCEALED IN WALL OR CEILING BRANCH CIRCUIT RACEWAY - CONCEALED IN FLOOR OR UNDERGROUND

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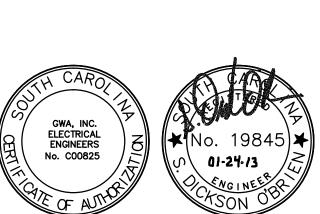
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OTE: ALL DEVICES SHOWN ON THIS SCHEDULE ARE SYMBOLIC ONLY. SEE ELECTRICAL SPECIFICATIONS FOR EXACT DEVICE REQUIREMENTS AND PERFORMACE CHARACTERISTICS

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GROUNDING TYPE BUSHING - ENCLOSURE - UNSPLICED BARE COPPER BONDING JUMPER. ← GROUNDING LUG ← INSTALL IN ACCORDANCE WITH NEC 250, SEE NOTES TO THIS DETAIL. INSULATING SLEEVE-PROVIDE MECHANICAL CABLE LUG. SIZE LUG TO SUIT CONDUCTOR. SHEET METAL SCREWS SHALL NOT BE USED TO CONNECT GROUNDING CONDUCTORS TO ENCLOSURES. NOTES TO THIS DETAIL GROUNDING TYPE -1. SIZE — EQUIPMENT BONDING JUMPER ON <u>SUPPLY SIDE</u> OF SERVICE: THE BONDING JUMPER SHALL NOT BE SMALLER THAN THE SIZES BUSHING SHOWN IN TABLE 250.66 FOR GROUNDING ELECTRODE SET SCREW - TIGHTEN-SECURELY AGAINST 2. SIZE — EQUIPMENT BONDING JUMPER ON <u>LOAD SIDE</u> OF SERVICE: THE BONDING JUMPER SHALL NOT BE SMALLER THAN THE SIZES RACEWAY THREADS. SHOWN IN TABLE 250.122 FOR EQUIPMENT GROUNDING RACEWAY -3. FOR MORE THAN ONE RACEWAY IN ENCLOSURE, BOND ALL LUGS TOGETHER BEFORE TYING BONDING JUMPER TO ENCLOSURE.

> GROUNDING OF CONDUITS 1" C. AND LARGER
>
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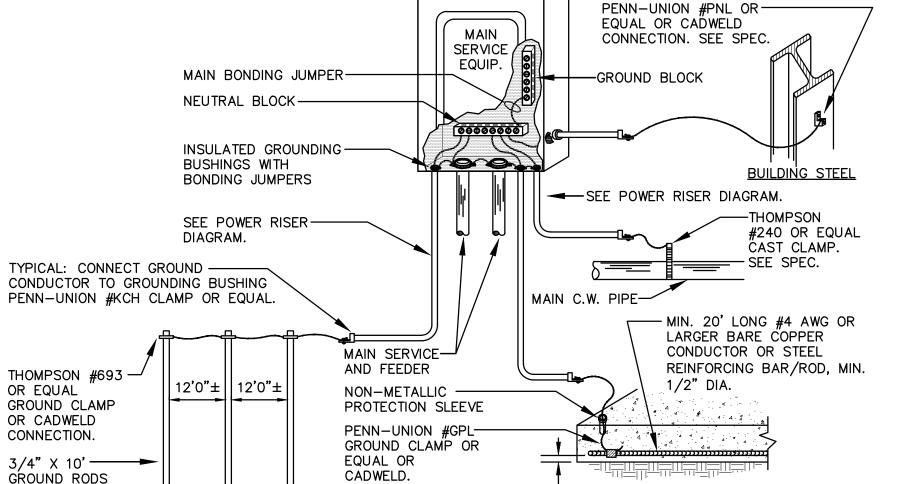




Midlands Office - Main 168 Laurelhurst Avenue Columbia, SC 29210 (803)252-6919 Fax (803)799-5494 gwa@gwainc.net

http://www.gwainc.net

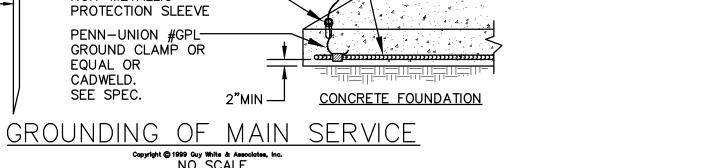
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SEE SPEC.

DRIVEN OUTSIDE

BUILDING.



LIGHTING FIXTURE SCHEDULE

SYMBOL	DESCRIPTION	MANUFACTURER	CATALOG NUMBER	VOLTAGE	LAMPS*	BALLASTS*	FIXTURE WATTAGE	NOTES
SA	LED HANDRAIL ASSEMBLY	INTENSE LIGHTING COLE	IVR2 SERIES (CUSTOM) STAINLESS STEEL-WALL MOUNT. HO LED-30° AYM-35 EQUAL	120V	LED	N/A	550W	LED ILLUMINATED HANDRAIL SYSTEM WITH ALTERNATING WALL MOUNT BRACKETS AND POST MOUNT POWERED BALUSTERS. SEE ARCHITECTURAL PLANS FOR MOUNTING. AT CONTRACTORS OPTION, REMOTE POWER SUPPLIES MAY BE UTILIZED IN LIEU OF POWER BALUSTER.
SB	LINEAR LED SIGN UPLIGHT	LUMENPULSE  COLOR KINETICS	LOGHRO-*-48-35K-* -*-UMAS-*-NO-ETE EQUAL	120V	LED	N/A	125W	SURFACE MOUNT AT BOTTOM OF SIGNAGE, AIM TO EVENLY ILLUMINATE SIGN. MOUNT AS CONTINUOUS ROW, LENGTH OF FIXTURES TO SUIT. FINISH AS DIRECTED BY ARCHITECT. PROVIDE OPTICAL DISTRIBUTION TO SUIT FINAL SIGN LAYOUT AND FIXTURE MOUNTING OFFSET.
SC	CAMPUS STANDARD LED POST TOP	STERNBERG	6ARC45T5 508 PEC1 (FIXTURE) 4212-FP5 (POLE)	120V	LED	N/A	110W	MOUNT TO CONCRETE BASE. PROVIDE POLE EPA RATED 90MPH (1.3 GUST FACTOR). PROVIDE FINISH TO MATCH EXISTING CAMPUS STANDARD. INTEGRAL PHOTOCELL.
SD	LED DOWN-LIGHT	LITHONIA H.E. WILLIAMS	DOM8 LED 35K EQUAL	120V	LED	N/A	27.5W	RECESSED - WET LOCATION TYPE.
A	4' UTILITY STRIP (2 LAMP)	LITHONIA H.E. WILLIAMS	UN-232-2GCUN EQUAL	120V	2-F032/835XP/ ECO (OCTRON T8)	OSI #QT-2x32T8/ISL (0.77 BALLAST FACTOR)	51W	SURFACE—CEILING. PROVIDE WITH WIREGUARD. COORDINATE LOCATION WITH EQUIPMENT LAYOUTS FOR UNIFORM LIGHTING LEVELS.

#### **NOTES TO LIGHTING FIXTURE SCHEDULE**

L1. LOCATE ALL FIXTURES IN STRICT ACCORDANCE WITH ARCHITECTURAL AND LANDSCAPE PLANS.

# EQUIPMENT AS OBVIOUSLY INDICATED AND COORDINATE WITH OTHER TRADES TO UTDOOR LIGHTING STANDARD & FIXTURE OWNLIGHT FIXTURE **TRANSFORMER**

PANELBOARD FLUSH JUNCTION BOX CEILING (()—I WALL)

PULL BOX OR JUNCTION BOX IN FLOOR DUPLEX RECEPTACLE ( HIGH MOUNT)

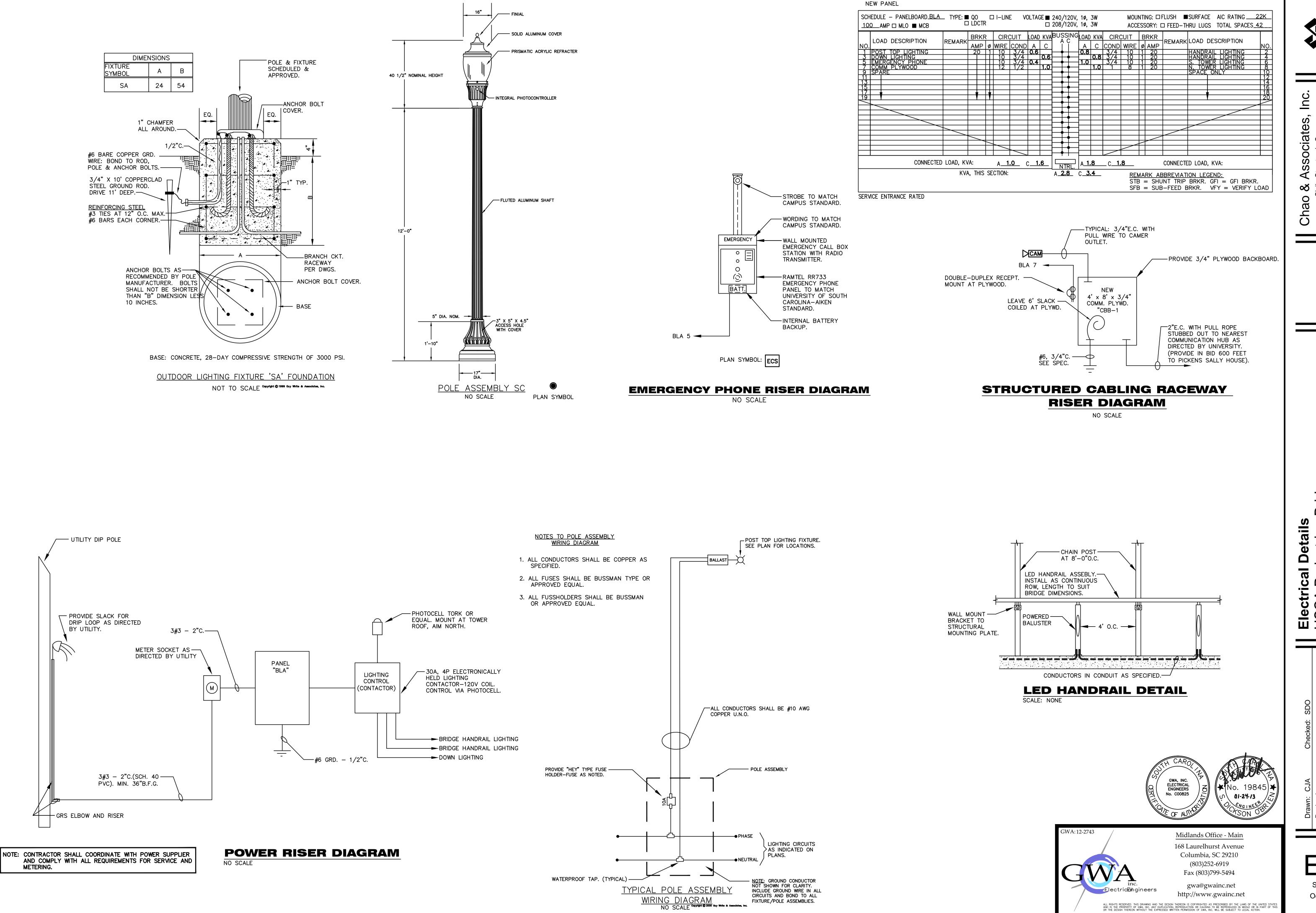
OGFI GROUND FAULT INTERRUPTER RECEPTACLE

MOTION SENSOR SWITCH

BRANCH CIRCUIT RACEWAY - EXPOSED

4. BOND BOTH ENDS OF EACH CONDUIT 1"C. AND LARGER.

5. EXTEND BONDING JUMPER TO EACH PANEL INCLUDING TWO-SECTION PANELS WITH FEED-THRU NIPPLES.



AND SURVEYORS

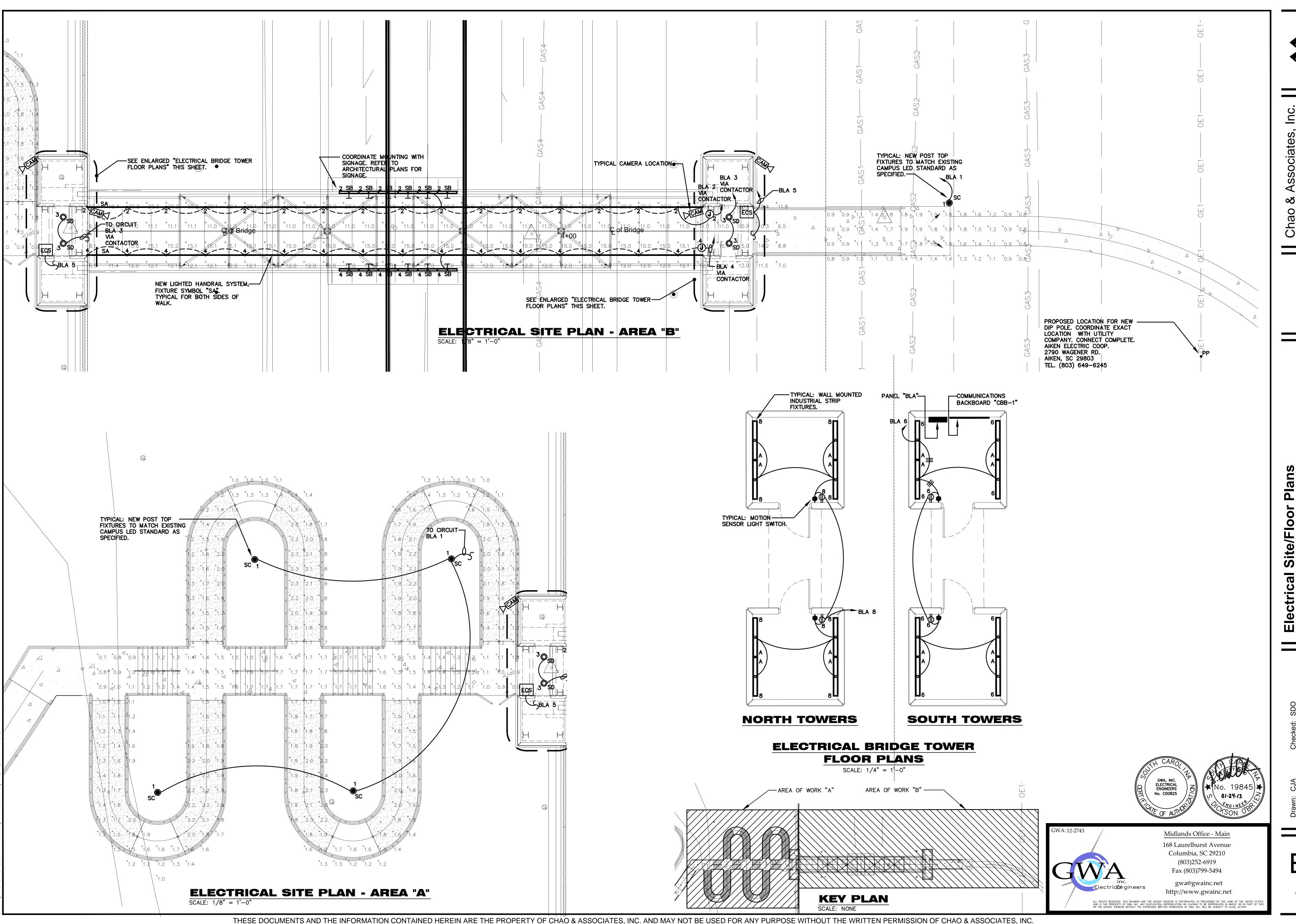
& LAND SURVEYORS

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